US EPA RECORDS CENTER REGION 5

ENVIRONMENTAL ASSESSMENT REPORT

ARLINGTON HEIGHTS LANDFILL Arlington Heights, Illinois

REPORT

Table of Contents

		Page
INTR	ODUCTION	1
	Purpose and Scope Site Location and Surface Conditions	1
POTE	NTIAL ENVIRONMENTAL CONCERNS	2
SITE	INVESTIGATIONS	2
	Data Review Site Inspection Drilling and Soil Sampling Air Monitoring Field Testing and Potentiometric Data Laboratory Soils Testing Groundwater and Surface Water Sampling and Analysis	3 3 5 5 5 5
REGI	ONAL GEOLOGY AND GROUNDWATER USE	6
SITE	CONDITIONS	7
	Site Geology and Hydrogeology	7
	Soil Units Groundwater Occurrence Hydraulic Conductivity Groundwater Flow Groundwater Quality	7 7 8 8 9
	Landfill Cap Landfill Gas IBT Vaults	9 11 11
CONC	LUSIONS AND RECOMMENDATIONS	12
EXHI	BITS	
APPE	NDICES	

List of Tables

- 1. Monitoring Well Data
- Results of Field Hydraulic Conductivity Tests in Monitoring Wells
- 3. Results of Laboratory Chemical Analysis Monitoring Wells
- 4. Comparison of Laboratory Chemical Analysis, Monitoring Wells vs Surface Water Pond
- 5. Volatile Organic Analyses Leachate and IBT Vaults

List of Exhibits

- 1. Site Location Map
- 2. Monitoring Well and Boring Location Map
- 3. Generalized Geologic Cross-Section A-A'
- 4. Generalized Geologic Cross-Section B-B'
- 5. Generalized Geologic Cross-Section C-C'
- 6. Generalized Potentiometric Contours

List of Appendices

- A. Boring Logs and Well Construction Sketches
- B. Laboratory Soils Test Reports
- C. Laboratory Groundwater Analysis Reports

INTRODUCTION

Purpose and Scope

The Village of Arlington Heights is owner/operator of a non-hazardous sanitary landfill at the corner of Kennicott Avenue and Nichols Road. The landfill accepted municipal refuse from about 1964 until about 1974 and since then has accepted only demolition debris. The Village plans to close the landfill with intended end use as a nine-hole golf course, to be developed and administered by the Park District. Harza Environmental Services, Inc. (HES) was retained to perform two concurrent studies in relation to this intended end use:

- o Develop a closure and post-closure care period plan for the landfill; and
- o Identify environmental issues of actual or potential concern to the Village as owner/operator.

This report presents the results of the environmental investigations completed in accordance with HES' proposal of February 29, 1988.

Site Location and Surface Conditions

The Arlington Heights Landfill site occupies about 52 acres at the southwest corner of Kennicott Avenue and Nichols Road in northwest Cook County, Illinois (Exhibit 1). The portion of the site used for sanitary waste disposal occupies approximately half this area along the east and north sides. The present landform rises about 45 feet above the adjacent ground surface in the east and 10 to 15 feet in the north.

There are several small buildings in the south part of the site including an office and a fire-training facility. A 6-million gallon on-ground water tank is located in the southwest part of the site. A well owned by the Village is located adjacent to the tank as is a monitoring well installed by the Illinois EPA. Present disposal of demolition debris is in the northwest and north-central parts of the property, near a small pond.

Adjacent land use is residential and industrial or commercial. Areas north of Nichols Road and east of the north part of Kenicott Avenue are single-family residential, including new development construction. West of the site, new construction for light industry is currently underway. South of the site, land use includes parking lots and a large industrial facility owned by Honeywell. A shopping center is located about 1,500 feet southeast of the site.

POTENTIAL ENVIRONMENTAL CONCERNS

The environmental study was directed at overall characterization of site conditions and at specific issues identified as potential concerns. Specific potential concerns are summarized below:

- o Groundwater Quality. The Illinois EPA maintains a ground-water monitoring well in the southwest part of the site and the IEPA facility file does not indicate contamination of groundwater. However, a major objective of the present study was to determine the occurrence of groundwater beneath the site, identify groundwater usage patterns, establish basic groundwater quality, and evaluate whether groundwater quality had been impacted.
- <u>Leachate Generation/Migration</u>. Generation and migration of leachate is a common concern at landfills. At this site, a leachate seep was discovered and repaired in 1984, suggesting that at least some leachate generation has occurred. In this program, observations during drilling and data from groundwater sampling and analysis were evaluated from the perspective of identifying whether leachate generation/migration was of major concern.
- <u>Landfill Gas.</u> Generation of landfill gas, primarily methane, is common at sanitary landfills due to decomposition of organic material. The current program aimed at identifying whether gas generation presently is a significant concern and, if so, assess the potential for off-site migration of gas.
- by the Village from Illinois Bell Telephone Company (IBT) concerning apparent contamination of several IBT vaults along the east site boundary. Contaminants reportedly included several potentially hazardous organic compounds. Although full evaluation of this condition was not within the scope of the present study, the collected data were examined to determine whether the contamination could be related to the landfill.

SITE INVESTIGATIONS

Site investigations were completed between April 4 and May 13, 1988 and included:

- o Data Review;
- o Site Inspection;
- o Drilling and Soil Sampling;
- o Field Testing;
- o Collection of Potentiometric Data;
- o Groundwater Sampling and Analysis; and
- o Laboratory Soils Testing

These activities are described in the following paragraphs.

<u>Data</u> <u>Review</u>

Information available on site and area conditions were reviewed prior to the field work and included regional and site specific data. Regional data included:

- o Geologic and hydrogeologic publications.
- o Water well logs from the Illinois State Geological Survey.
- o USGS Topographic Quadrangle maps.

Site specific data included:

- o Site boring logs (Novak, Dempsey & Associates, 1972).
- o Results of leachate analysis (1984).
- o Orthophotomaps (1974)
- o Illinois EPA records on the landfill operation.
- o Results of chemical analysis, IBT vaults.

Additional information was obtained in discussions with Village personnel knowledgeable about the landfill history and operation.

Site Inspection

The site was inspected to locate surface features and potential drilling locations, to identify surface evidence of leachate or landfill gas migration, if any, and to characterize adjacent land use.

Drilling and Soil Sampling

Eight groundwater monitoring wells were installed to characterize subsurface soil units and groundwater flow patterns. Geologic data from these wells were supplemented by data from borings prepared in 1972 by Novak, Dempsey and Associates, Inc. and made available to HES by the Village. Four shallow soil borings also were completed to evaluate the thickness and properties of the existing landfill cover. Monitoring well and boring locations used in the evaluations are shown on Exhibit 2 and well data are summarized in Table 1.

Drilling was performed by Testing Service Corporation (TSC) of Carol Stream, Illinois, using 7-inch diameter hollow stem augers. Soil samples were obtained continuously using a 2-inch diameter split spoon sampler and Standard Penetration Test (SPT) methods (the sampler was driven 24-inches for each sample rather than the 18-inches used in the SPT method). A HES hydrogeologist monitored the drilling, prepared geologic logs, and evaluated the boring data. Field classification os soil samples used the Unified Soil Classification (USC) System. Geologic logs are presented in Appendix A.

TABLE 1
Monitoring Well Data

Well/Boring Number	Ground Elevation	Top of Casing Elevation	Drilled Depth	Screen <u>Depth</u>
	(1)	(1)	(ft)	(ft)
MW-1	112.18	114.26	40.0	33.5-38.5
MW-2	88.83	90.41	20.0	8.6-13.6
MW-3	92.67	94.47	24.0	10.7-15.7
MW-4	99.62	102.52	34.0	26.9-31.9
MW-5	86.52	89.02	22.0	16.5-21.5
MW-6	120.15	123.15	46.0	38.8-43.8
MW-7	113.15	115.40	48.0	42.6-47.6
MW-8	98.65	101.10	24.0	17.4-22.4

NOTE: Elevations are relative to site datum - 100 ft at the fire hydrant in the northeast corner of the site.

Monitoring wells were constructed with 2-inch diameter PVC casing, 5-foot long, 10-slot PVC well screen, sand pack, bentonite pellet seals, Volclay High Solids Grout, surface concrete and lockable steel surface protectors. Construction sketches are included in Appendix A. Wells were developed by bailing which continued until the discharge water was stable with respect to pH, temperature, and conductivity. Several wells remained turbid due to poor recharge characteristics.

The ground elevation, top of casing elevation (monitoring wells), and location of each well and boring were surveyed by TSC after completion. Because no bench marks are available on-site, a fire hydrant at the northeast corner of the site was used as site datum.

Air Monitoring

Air monitoring was performed during all field activities both as a health and safety precaution and to provide supplementary information on soil or groundwater contamination or landfill gas generation. Monitoring was performed using an HNu photoionization detector (PID) equipped with a 11.7 eV probe and with an explosive gas meter. Readings are included on boring logs in Appendix A.

Field Testing and Potentiometric Data

Variable head hydraulic conductivity tests were performed in each well. Three wells (MW-2, MW-6, and MW-8), recharged too rapidly for reliable measurements and these could not be analyzed. Groundwater levels were measured in the wells after installation and before the hydraulic conductivity tests and were used to assess groundwater flow directions and rates.

Laboratory Soils Testing

Selected soil samples from the drilling were tested by TSC for physical properties. Tests further characterized the site subsoils and provided data for engineering studies. Tests included classification, water content, gradation (sieve), liquid and plastic limits, unconfined compressive strength, dry density, and optimum moisture. Tests reports are presented in Appendix B.

Groundwater and Surface Water Sampling and Analysis

Groundwater samples were collected from the monitoring wells using a hand bailer after purging of the wells. A surface water sample also was collected from the pond in the northwest part of the site. Sampling equipment was decontaminated with steam between each use. Samples were analyzed to characterize water chemistry and indicate contamination. Parameters analyzed included pH, specific conductance, total organic carbon (TOC), total organic halogens (TOX), alkalinity (bicarbonate), chloride, sulfate, calcium, magnesium, potassium, and sodium. Analyses were per-

formed by NET-Midwest of Bartlett, Illinois. Laboratory reports are presented in Appendix C.

REGIONAL GEOLOGY AND GROUNDWATER USE

The site area is underlain by thick glacial drift deposits resting on Silurian dolomite bedrock. The glacial drift is approximately 150 feet thick and consists primarily of clayey soils (till) deposited by the glaciers. The tills contain intermittent seams and interbeds of sand and gravel. The drift, collectively, is of Wisconsin age and is assigned to the Woodfordian Substage. A number of sand pits are located on and near the site, suggesting the presence of shallow sand deposits in the site area.

Groundwater is an important resource in northern Illinois. There are three major groundwater aquifers commonly used for water supply. These are:

- o Cambrian-Ordovician aquifers;
- o Silurian dolomite aquifer; and
- o Glacial drift aquifers.

The Cambrian-Ordovician aquifers are sandstone bedrock units which occur at depths of 500 to 2,000 feet below the surface. These can provide large quantities of groundwater to wells and often are used for municipal or industrial water supply. Yields often are 1,000 gallons per minute or more. There are two wells in this aquifer within about a half mile of the site. However, because of their depth, these aquifers have little bearing on the site investigation.

The Silurian dolomite is a major bedrock aquifer in most of northern Illinois and, depending on its local properties, can provide sufficient groundwater for municipal and industrial supply. In the site area, it is more commonly used for domestic supply at yields generally between 10 and 25 gallons per minute. There are at least 7 dolomite wells within about one-half mile of the site, including a well owned by the Village within the site. The top of the dolomite is estimated to occur at depths in excess of 150 feet based on the well records In the area, and the piezometric surface typically occurs at depths between about 75 and 100 feet. Regionally, groundwater flow in the dolomite is controlled by major surface drainages and areas of concentrated pumping withdrawal. In the site area flow in the dolomite is estimated to be generally eastward, toward the DesPlaines River.

The glacial drift is a variable, but important, groundwater resource in northern Illinois, although it is not used near the site. The clayey till which comprises most of the drift in northern Illinois generally contains little free groundwater and is not a significant resource. However, sand and gravel units in the till can contain substantial groundwater and, if continuous and extensive, can supply water to domestic and, locally, municipal or industrial wells. The most productive sands and gravels frequent-

ly underlie major drainages or immediately overlie bedrock. Where overlying the dolomite bedrock, the two aquifers are in hydraulic communication. The piezometric or water table depth in the drift cannot be determined on a regional basis because of the variability of the unit. In general, groundwater flow will parallel topography but can vary in any specific area. Groundwater also occurs at shallower depths in the drift, generally in localized sand and gravel seams such as encountered at this site. These shallow water-bearing zones may be used for domestic water supply, but are variable in occurrence and properties and generally are not suitable for more extensive development. There are no records of water supply wells in the drift within about 1/2 mile of the landfill.

SITE CONDITIONS

Site Geology and Hydrogeology

<u>Soil Units</u>. The site drilling program investigated subsurface soil conditions to depths of 22 to 48 feet around the site perimeter. These data were supplemented by review of boring logs prepared by Novak, Dempsey and Associates, Inc. in 1972, some of which extended to greater depths. Soil units typically encountered in the monitoring wells are illustrated by geological cross-sections A, B, and C on Exhibits 4, 5, and 6.

All monitoring well borings penetrated topsoil and were completed in the underlying glacial drift. The borings were not designed to penetrate bedrock, which is estimated to occur at a depth of 150 feet under the site.

The topsoil ranged in thickness from about 2 inches in well MW-4 to 24 inches in MW-1 and MW-6 and consists of brown and yellow clay with traces of sand, silt and roots. The topsoil is underlain by glacial drift consisting predominantly of dense clay (CL in the Unified Soil Classification System), with varying quantities of silt, sand, small and large gravel and, locally, cobbles. Silt seams were encountered locally. Within the clayey till, the wells penetrated a strata of multicolored, fine- to coarse-grained sand and gravel. The top of this unit ranged in depth from 6 feet in MW-2 to 44 feet in MW-7. This unit comprises a shallow groundwater aquifer and appears to be continuous under the site. varies widely in thickness from only 1.5 feet in MW-4 to more than 12 feet in MW-1 and MW-8, where the bottom was not encountered. Some of the 1972 boring logs suggest that the sand may attain local thicknesses of 36 feet. In most wells and borings, the sand was underlain by clays similar to those above.

Groundwater Occurrence. Groundwater was encountered in all monitoring wells, primarily in the shallow sand aquifer. All of the monitoring wells were screened in this zone. In most wells, this shallow sand unit is fully saturated and the groundwater is confined, groundwater levels rising above the top of the water bearing strata. In wells MW-1 and MW-6, the upper part of the

sand was not saturated and, in these areas, the aquifer is unconfined and the measured water levels represent the top of the saturated zone (e.g. "water table" conditions). This is due to variations in groundwater elevations and in the elevation and thickness of the water-bearing zone across the site.

<u>Hydraulic</u> <u>Conductivity</u>. Results of field permeability tests performed in the monitoring wells are summarized in Table 2.

TABLE 2

Results of Field Hydraulic Conductivity Tests in Monitoring Wells

Well Number	Estimated <u>Hydraulic Conductivity</u> 0.09 ft/min
2	(1)
3	0.01 ft/min
4	0.02 ft/min
5	0.05 ft/min
6	(1)
7 .	0.05 ft/min
8.	(1)

NOTE: (1) Recharge too rapid for analysis.

Since all of the wells are screened in the shallow sand zone, the test results are representative of this unit and indicate an average estimated field hydraulic conductivity of about 0.04 ft/min (2.18 X 10-2 cm/sec). Tests were not performed in the clayey portions of the glacial drift, but these materials typically have field permeabilities of 1 X 10-6 or 1 X 10-7 cm/sec.

<u>Groundwater Flow</u>. The depth to groundwater was measured in each monitoring well and the data converted to relative potentiometric elevations. Potentiometric elevations were used to generate potentiometric contours on Exhibit 6.

The potentiometric contours indicate the pattern of groundwater movement in the shallow sand aquifer at the site. Groundwater flow patterns are relatively complex, reflecting the variable geometry, properties and thickness of the water-bearing unit. Typical of many shallow drift sands, there are several groundwater flow divides and overall flow patterns in adjacent areas are not clearly defined. Principal subsurface inflow to the site occurs in the vicinity of MW-3 in the southwest, MW-2 in the northwest and, possibly, MW-1 on the east. Principal discharge areas are near MW-5 on the west and MW-6 and MW-7, in the southeast. Additional discharge may occur under part of the north boundary at wells MW-4 and MW-8. In general, wells MW-2 and 3 are upgradient wells and MW-5, MW-6, and MW-7 are downgradient.

Hydraulic gradients in the sand are variable across the site

because of the relatively complex flow pattern. Calculated gradients range from about 0.003 ft/ft to 0.012 ft/ft in different areas. Using Darcy's Law in the form of v = ki/n, where "v" the average groundwater flow velocity in the sand, "k" is the average hydraulic conductivity (0.04 feet/min from the field tests), "i" is the hydraulic gradient (described above), and "n" is the effective porosity (estimated at 30 percent), groundwater flow velocities in different site areas are estimated to range between about 0.6 and 2.3 feet/day.

The shallow groundwater zone at the site probably is hydraulically connected to surface water in the pond in the northwest part of the site. This connection results in recharge of the groundwater by infiltration from the pond which further distorts local groundwater flow patterns. Data are not sufficient to define this connection in detail.

Groundwater Quality. Results of analysis of groundwater samples are summarized in Table 3. Maximum, minimum and average analytical results for the groundwater samples are compared to results from the on-site pond in Table 4.

The parameters analyzed were selected to characterize overall groundwater chemistry and as indicators of possible groundwater contamination. In the present study, they were not intended to identify specific contaminants.

The analyses indicate that concentrations of most constituents are within the range of values expected for shallow groundwater in the region. Generally, wells MW-1, MW-3, and MW-8 have the highest concentrations of indicator parameters TOC, TOX, and specific conductance. Of these, MW-3 is clearly upgradient and unaffected by the landfill, while MW-1 and MW-8 are located such that impacts are possible, although not clearly defined. Evaluation of the analytical and groundwater data, however, suggests no pattern indicative of groundwater contamination due to the landfill.

Landfill Cap

Borings SB-1 through SB-4 were drilled to evaluate the thickness and properties of the existing landfill cap. Borings SB-1 and SB-2 indicate that the cap in the north part of the landfill is 3 to 3.5 feet thick while borings SB-3 and SB-4 indicate that the cap in the east part of the landfill is 11 and 9.5 feet thick, respectively. Field classifications and laboratory tests on samples from the borings indicate that the cap materials are organic and inorganic clays (OL, OH and CL in the Unified Soil Classification System) with dry densities ranging from 84.8 to 118.8 lbs/cubic ft. Further results of physical properties tests on the cap material are presented in Appendix B.

TABLE 3

Results of Laboratory Chemical Analysis - Monitoring Wells

•		Con	centra	tion/Mo	nitoring	y Well	Number		•
PARAMETER	MW-1	MW-2	MW-3	MW-4	MW-5	MW-6	MW-7	MW-8	Unit
Bicarb-CaCO3	412	476	. 478	294	138	388	230	680	mg/l
Chloride	112	140	254	82	24	182	128	.210	mg/l
Conductivity	1540	1770	2530	1100	934	1430	1040	2200	umhos/cm
рН	7.42	7.43	6.93	7.42	7.90	7.16	7.43	6.85	Units
Sulfate	219	205	369	131	239	84	. 75	59	mg/l
TOC	4.7	4.4	12.2	2.0	2.4	2.1	1.8	20.6	• .
TOX	0.080	0.013	0.093	<0.005	<0.005	0.009	0.018	0.056	mg/l
Calcium	159	118	238	89.3	81.6	137	93.8	157	mg/l
Magnesium	68	85	90	47	26.7	48	48.3	66	
Potassium	7.58	26.2	5.33	3.95	3.25	5.35	4.79	37.2	-
Sodium	52.5	83	161	27.6	62	61	46.3	163	mg/l
*	======	=====:	=====:			=====:			======

TABLE 4

Comparison of Laboratory Chemical Analysis,
Monitoring Wells vs Surface Water Pond

G	roundwa	ter	Pond	
MAX	MIN	AVG(1)	Sample	Unit
680	138	387	138	mg/l
254	24	142	. 72	mg/l
2530	934	1568	659	umhos/cm
7.90	6.85	7.32	8.15	Units
369	59	173	62	mg/l
20.6	1.8	6.3	5.1	mg/l
0.093	<0.005	0.034	0.006	mg/l
238	81.6	134	59.7	mg/l
90	26.7	60	21.4	mg/l
37.2	3.25	12	3.9	mg/l
163	27.6	82	39.3	mg/l
	MAX 680 254 2530 7.90 369 20.6 0.093 238 90 37.2	MAX MIN 680 138 254 24 2530 934 7.90 6.85 369 59 20.6 1.8 0.093 < 0.005 238 81.6 90 26.7 37.2 3.25	680 138 387 254 24 142 2530 934 1568 7.90 6.85 7.32 369 59 173 20.6 1.8 6.3 0.093 <0.005 0.034 238 81.6 134 90 26.7 60 37.2 3.25 12	MAX MIN AVG(1) Sample 680 138 387 138 254 24 142 72 2530 934 1568 659 7.90 6.85 7.32 8.15 369 59 173 62 20.6 1.8 6.3 5.1 0.093 <0.005

⁽¹⁾ Average of all available analytical results.
< = Parameter not detected at indicated limit.</pre>

Landfill Gas

Landfill gas was encountered during drilling of wells MW-1 and MW-6 along the southeast site perimeter based on results of air monitoring and field observations. The upper part of the shallow sand zone is not saturated in this local area, providing a potential gas migration pathway in the unsaturated zone. In other site monitoring wells, the sand is fully saturated, not providing a pathway for gas migration. Landfill gas also was detected in landfill cap borings SB-3 and SB-4, indicating that some gas is being generated, but is generally contained by the present cap. The site inspection did not indicate evidence of vegetative stress which often is an indicator of landfill gas. Evaluation of all data indicates that, although landfill gas is being generated to some degree, the natural clayey soils and saturated sands control lateral migration except at MW-1 and MW-6, and that the existing cap effectively controls upward migration. Landfill gas does not appear to represent a major problem for off-site migration, although provision for gas observation may be required in the closure plan.

IBT Vaults

In October 1987, IBT representatives met with Village personnel and notified the Village that inspection of a conduit system they had installed along the east site boundary was contaminated by various volatile organic compounds. IBT had observed possibly contaminated liquids in four vaults along the conduit system which are accessed by manholes. The vaults are shown approximately on Exhibit 2. Vault 1 is located south of the site entrance gate on Kennicott Avenue and vaults 2, 3, and 4 along the east site boundary.

IBT collected samples of the water in the vaults which were analyzed by MetPath-Teterboro Laboratory of New Jersey for various heavy metals and organic compounds. Results indicated variable concentrations of several compounds of potential environmental concern, summarized in Table 5. In August of 1984, IEPA also sampled and analyzed leachate from the seep identified and repaired at that time. For comparison, these results also are shown.

Except for benzene and toluene, the contaminants detected in the vaults and the leachate sample are chlorinated organic compounds used as solvents, cleaning or degreasing fluids, refrigerants, and in chemicals and plastics manufacturing, among other uses. They include several compounds of potential concern from environmental and human health viewpoints. For example, chlorinated organic chemicals tend to persist in the environment and are not readily biodegraded. Further, chloroform, chloromethane, 1,2-dichloroethane, methylene chloride, tetrachloroethene, trichloroethene, vinyl chloride and benzene are known or suspected human carcinogens.

- NOT

The highest contaminant concentrations and greatest number of different compounds were found in vaults 2 and 3 adjacent to the southeast side of the site. Lower concentrations and generally fewer different contaminants were found in vaults 1, south of the site, and 4, on the northeast corner. This pattern suggests the possibility that the contamination may have resulted from the landfill, although subsequent migration through the conduits or other pathway would then be required and other sources are equally possible (e.g. off-site spills or dumping). Comparison of analytical results from the vaults and the leachate seep indicate that, although there are similarities in the contaminants detected, there also are disparities and no reliable conclusion can be reached. Nonetheless, the nature of the contamination is such that the possibility of the landfill being the source should be examined.

If the landfill is the source of the contamination in the vaults, the contamination must derive from migration of leachate. For this to be the case, a migration pathway must exist either with groundwater in the shallow sand aquifer or in local permeable seams above the sand. Data from the monitoring wells show that the sand and potentiometric levels in the sand are below the vaults and, therefore, this possible pathway does not apply.

The possibility of leachate migration to the vaults in shallow zones above the sand aquifer cannot be resolved with the available data. Such zones can be quite limited in extent and, although not encountered in the drilling, could occur between the well locations. At the present time, the available data are sufficient only to say that no evidence of leachate migration was observed at the locations of the monitoring wells. However, this possibility must be considered until further data are available.

SUMMARY AND CONCLUSIONS

The environmental study of the Arlington Heights Landfill site indicates the following site conditions:

- o The site is underlain by approximately 150 feet of unconsolidated glacial drift sediments. The drift consists primarily of low permeability clay tills, but contains a sand and gravel layer at variable depths from 6 to 44 feet across the site. This seam is continuous under the site and comprises a shallow water-bearing unit.
- o The shallow groundwater is not used for water supply in the area.
- o Groundwater quality at the site generally is within normal parameters. The presence of halogenated organic compounds is indicated, but is not reflected in a pattern related to the landfill.
- o The presence of past leachate generation is indicated by a

Seasonal Champs, Sharry

TABLE 5. Volatile Organic Analyses - Leachate and IBT Vaults

	CONCE	NTRATION	(ug/1)		•
	Seep		IBT V	aults	
COMPOUND		#1	#2(5)	#3	#4
Chloroform	ND	ND	ND .	1.9	ND
Chloromethane (1)	ND	ND	ND	ND	ND
1,2-dichloroethane (2)	7.5	ND	15.8	17.4	10.9
Trans-1,2-dichloroethene (3)	ND	104	6224	291.4	78.3
1,2-dichloropropane (4)	ND	ND	79.2	9.9	ND
Trans-1,3-dichloropropene	ND	ND	384	ND	ND
Methylene chloride	ND	ND	ND	2.8	ND
tetrachloroethene	ND	ND	ND	4.3	ND
Trichloroethene	124	2.3	75.0	28.2	ND
Vinyl chloride	ND	12.2	370	42.6	8.1
Benzene	22.2	ND	ND	4.7	ND.
Toluene	270	ND	ND	ND	ND
Carbon Tetrachloride	600	ND	ND	ND	ND
1,1,1-trichloroethane	600	ND	ND	ND ·	ND
1,1-dichloroethane	700	ND	78.2	ND	ND
1,2-transdichloroethylene	20.7	ND	ND	ND	ND
1,2-Cis-dichloroethylene	1700	ND	ND	ND	ND

ND = Parameter not detected.
(1) Methyl chloride.
(2) Ethylene dichloride.
(3) Acetylene dichloride.

- . (2)

 - (4)
 - Propylene dichloride. Higher of two analyses. (5)

reported leachate seep in 1984. However, groundwater or surface water impacts are not indicated.

- o Landfill gas is being generated in unknown, but apparently not large quantities. The present landfill cap appears to control upward migration of gas, while the clayey soils and saturated sands control lateral migration in most areas. Locally unsaturated portions of the shallow aquifer allow lateral migration of gas in parts of the east site boundary.
- o Reported volatile organic contamination in IBT vaults along the east site boundary includes compounds of concern. Leachate migration from the landfill is one of several possible sources. The contamination is not due to leachate migration with groundwater in the shallow sand layer. However, the possibility of leachate migration in localized, shallower soil zones cannot be eliminated with the available data.

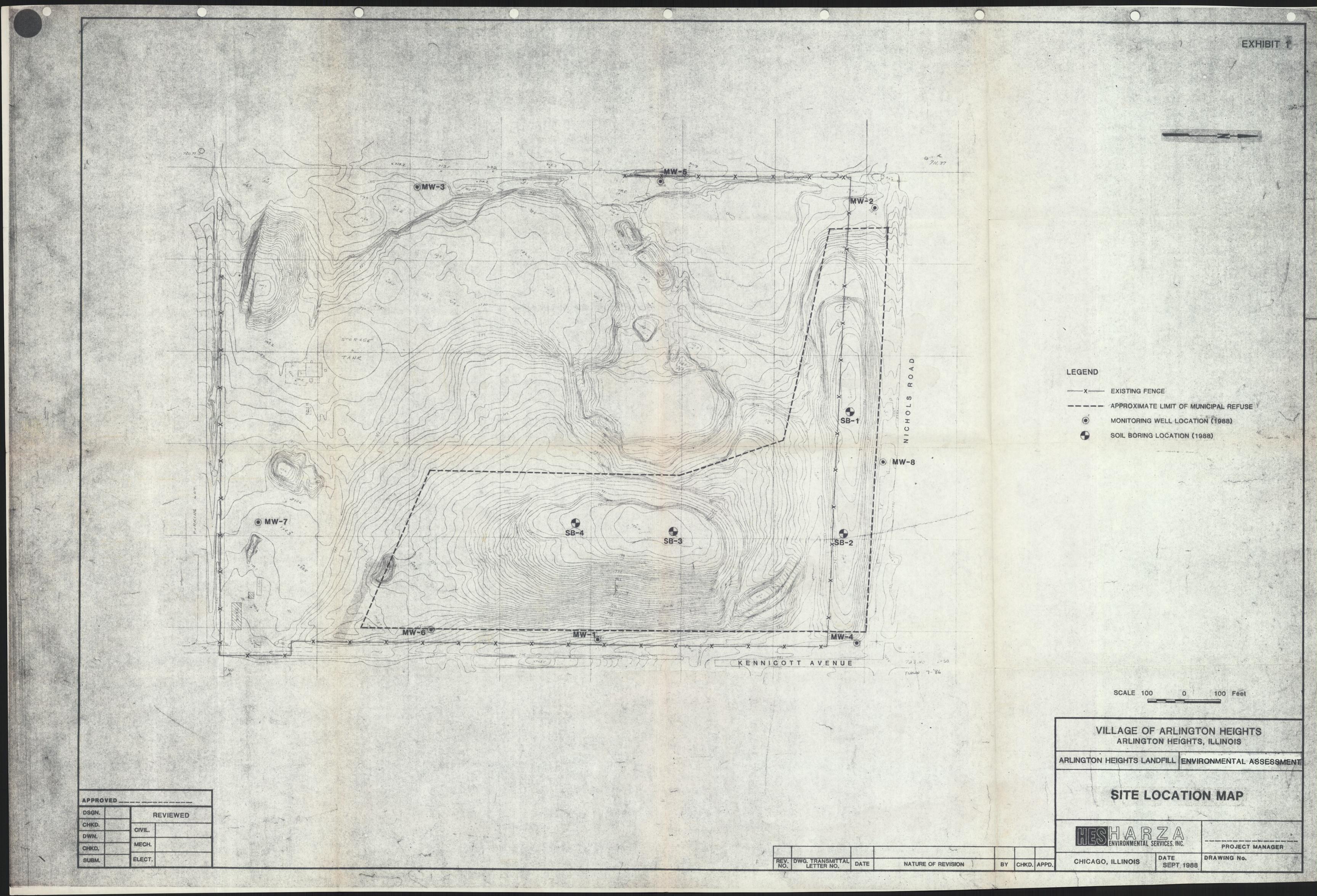
Results of site investigations do not indicate environmental problems which, in our judgement, cannot be adequately addressed in the closure and end use. The following activities are recommended to be addressed prior to or during implementation of the closure and end use plan.

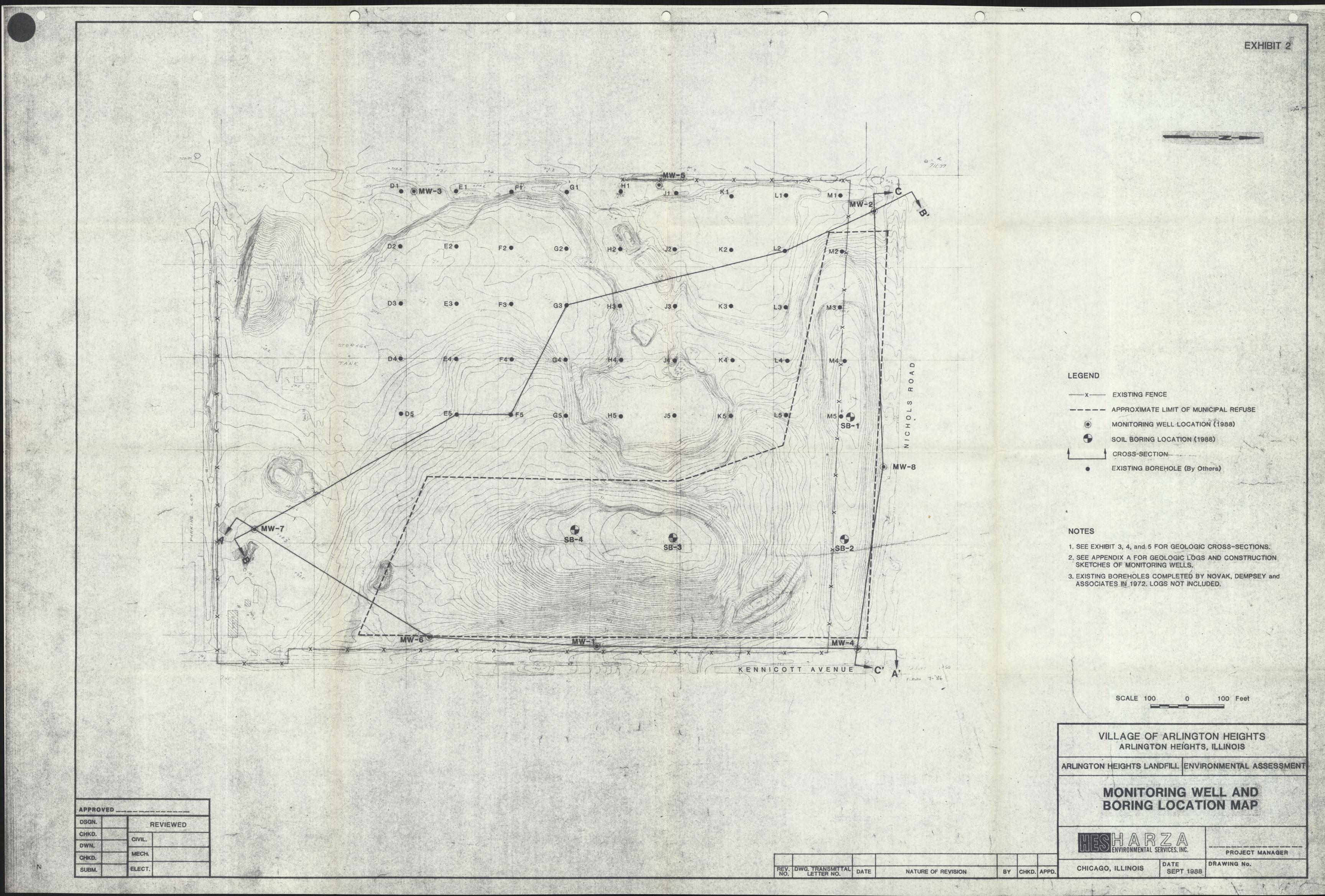
- o Contamination of the IBT vaults should be investigated to determine whether the landfill is the source of contamination. We recommend the following activities prior to or during implementation of the closure plan:
 - 1) The vaults should be inspected to establish present conditions (presence and nature of contamination, possible points of entry, apparent means of contaminant migration between vaults, etc.). If contamination is present, collect additional samples and analyze for compounds of concern using appropriate chain-of-custody and QA/QC protocols.
 - The Village should investigate the possibility that leachate is migrating from the landfill by completing a series of test pits along the east site boundary. Pits should be excavated to depths equivalent to the vaults. Evidence of leachate migration in the shallow soils should be determined visually and by air monitoring and soil samples should be collected and analyzed for contaminants of concern.
 - 3) The Village should sample and analyze groundwater from monitoring wells showing elevated TOX levels for specific organic and heavy metal contaminants. The wells nearest the vaults (MW-1, MW-4 and MW-6) should be included.

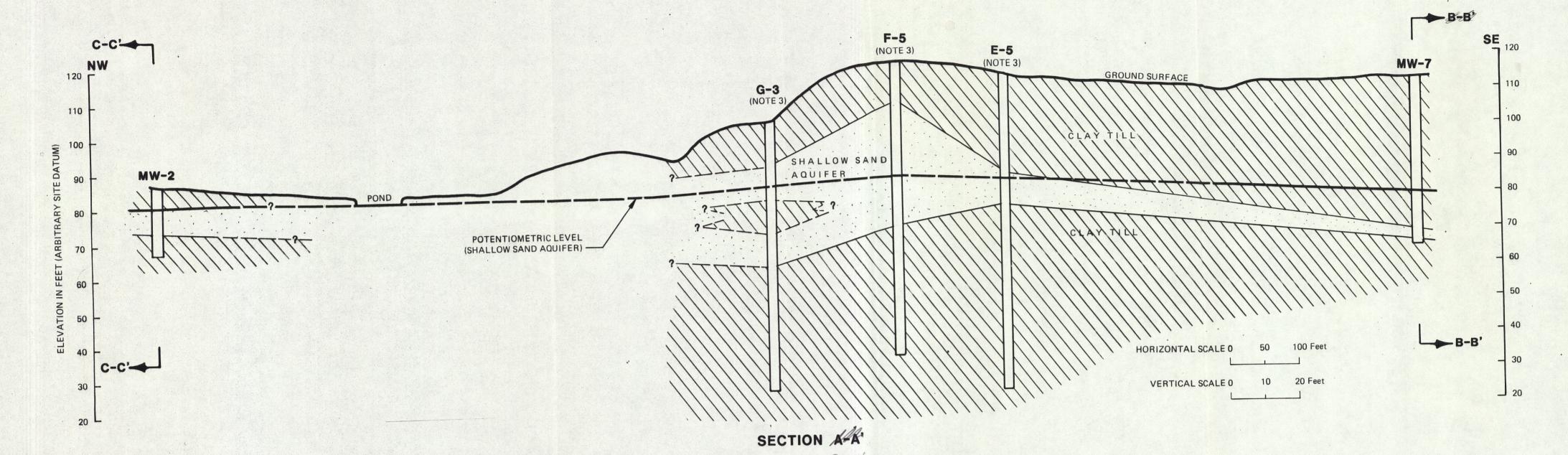
If the landfill is indicated to be the cause of contamination to the vaults, provision will be required in the closure plan for appropriate mitigation measures.

- o Implement groundwater monitoring using the wells installed in this program. Groundwater monitoring will be part of the post-closure care period plan.
- o Although not indicated to be a major problem, regular inspection for gas migration and provision for gas monitoring along the east side of the site is recommended.

EXHIBITS







NOTES

- 1. See Exhibit 2 for well and section locations.
- 2. See Appendix A for logs and construction sketches.
- 3. 1972 Boring data provided by Village of Arlington Heights.
 Drilling by Novak, Dempsey & Associates, Inc..

HESHARZA ENVIRONMENTAL SERVICES, INC.

LEGEND

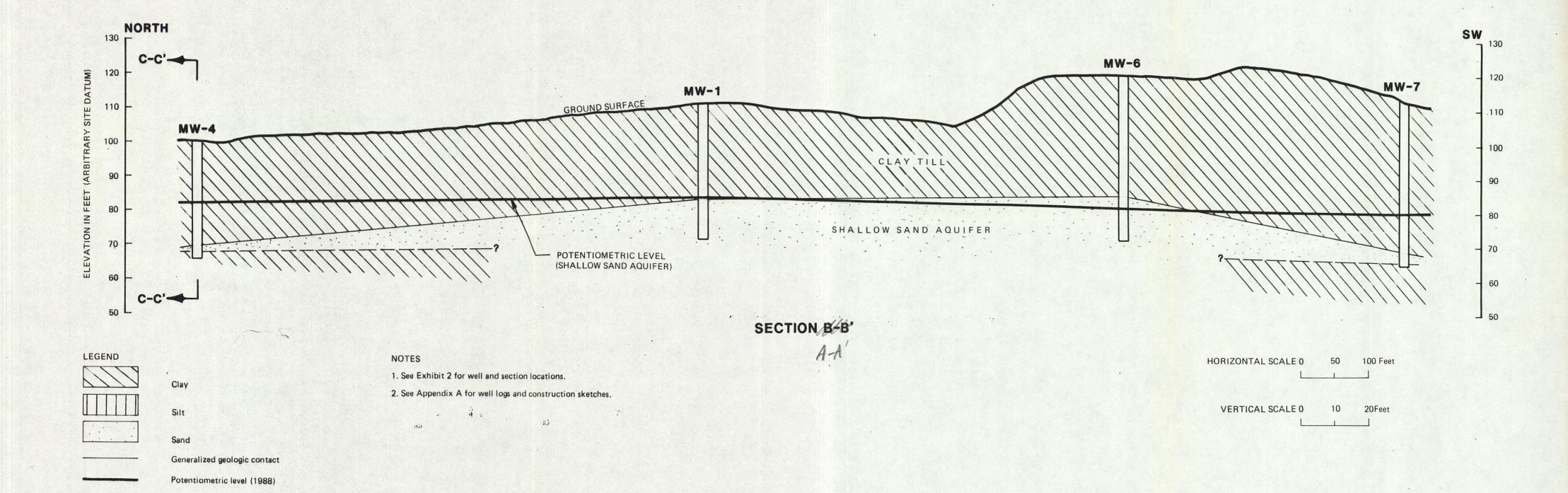
September 1988

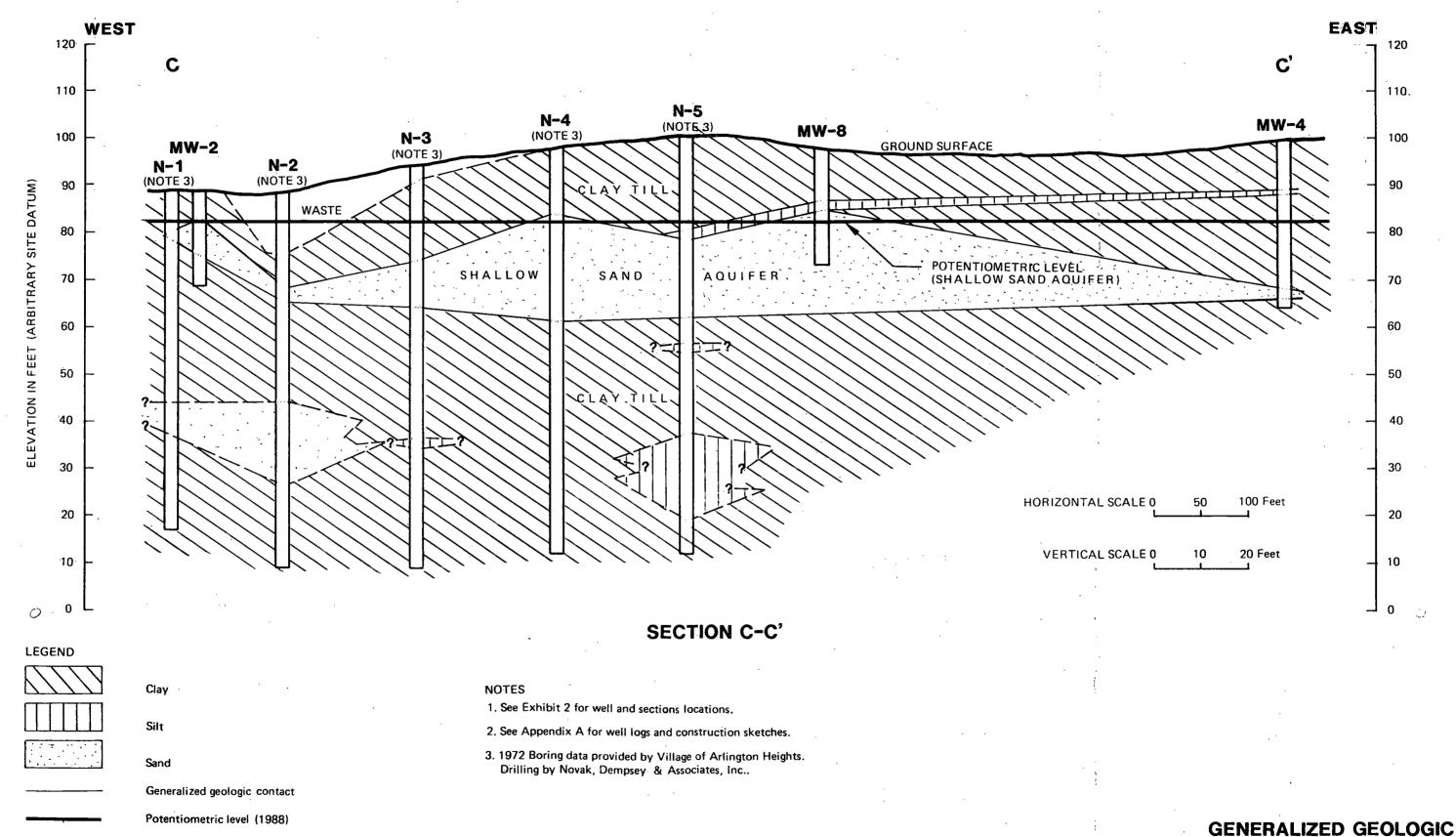
Generalized geologic contact

Potentiometric level (1988)

GENERALIZED GEOLOGIC
CROSS-SECTION AT RES

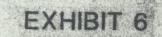
ARLINGTON HEIGHTS LANDFILL Arlington Heights, Illinois

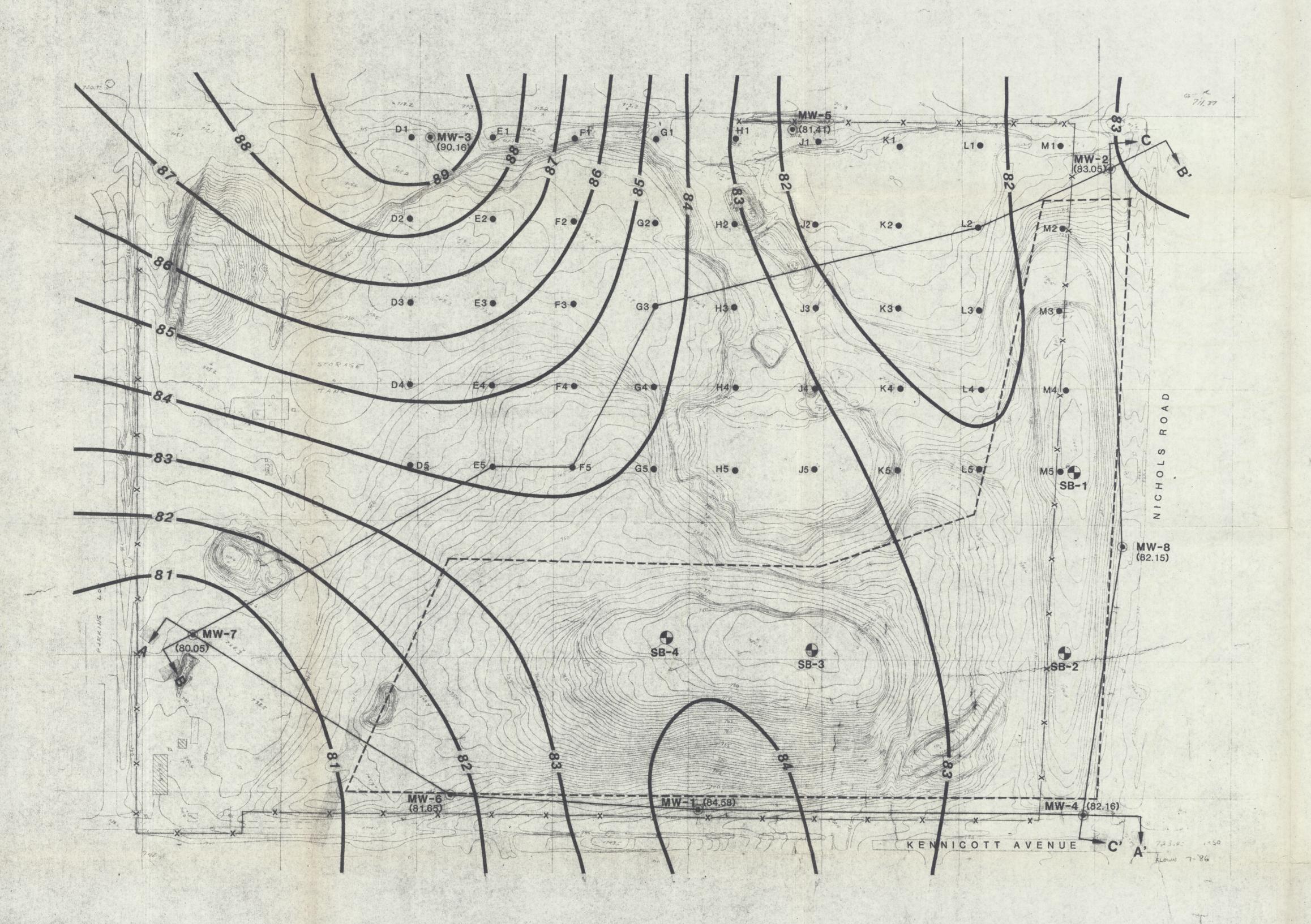




CROSS-SECTION C-C'

ARLINGTON HEIGHTS LANDFILL Arlington Heights, Illinois





LEGEND

---- APPROXIMATE LIMIT OF MUNICIPAL REFUSE

MONITORING WELL LOCATION (1988)

SOIL BORING LOCATION (1988) CROSS-SECTION

EXISTING BOREHOLE (By Others)

-89 - GENERALIZED PIEZOMETRIC CONTOUR

NOTES

- 1. SEE EXHIBIT 3, 4, and 5 FOR GEOLOGIC CROSS-SECTIONS.
- 2. SEE APPENDIX A FOR GEOLOGIC LOGS AND CONSTRUCTION SKETCHES OF MONITORING WELLS.
- 3. EXISTING BOREHOLES COMPLETED BY NOVAK, DEMPSEY and ASSOCIATES IN 1972. LOGS NOT INCLUDED.
- 4. PIEZOMETRIC ELEVATIONS BASED ON ARBITIARY SITE DATUM
 OF 100 AT TOP OF FIRE HYDRANT AT NORTHEAST CORNER
 OF SITE.

SCALE 100 100 Feet

VILLAGE OF ARLINGTON HEIGHTS
ARLINGTON HEIGHTS, ILLINOIS

ARLINGTON HEIGHTS LANDFILL ENVIRONMENTAL ASSESSMENT

GENERALIZED PIEZOMETRIC CONTOURS Shallow Sand Aquifier



PROJECT MANAGER DRAWING No. SEPT 1988

APPROVED											
DSGN.	REVIEWED										
CHKD.	CIVIL										
DWN.											
CHKD.	MECH.										
SUBM.	ELECT.										

REV. DWG. TRANSMITTAL DATE NATURE OF REVISION

CHICAGO, ILLINOIS BY CHKD. APPD.



Appendix A

Boring Logs and Well Construction Sketches

TSC

SOIL BORING LOG

ARLINGTON HEIGHTS GANDFILL

Location of Boring:	14
A 209064	760' 50'

Water Depth: 27. 55 27.6 5-13 1000a 1200p 38.52 38.0 Casing Depth

H.S. AUGER Drilling Method (s): _

Hole/Casing Size (s):

Boring No.: -EAST SIDE

Coordinates: N

Ground Elevation: _ Casing Elevation:

Total Depth:

Com	Date pleted:	4-3	27-8	Ź Tim	ne: _/	0	000	· _			C CASING/SCREEN Bottom						
ŧ	pth			ven	Length Recovered (in/cm)	Graphic Recovery	il ion	Log	Sampling Method (s): SPT 24" SPOON - CONTINUO SAMPUNG Sample Dimensions: 1,37/N Hammer Weight/Drop: 140/								
g Depth	le De	Sample No.	s per 15 cm	E C	# E	hic R	ed So ificati	hic L	Surfac	e Cond	ditions: GRASS						
Boring	Sample Depth (ft/m)	Samp	Blows per 6 in/15 cm	Length Driven (in/cm)	Leng (in/cr	Grap	Unified Soil Classification	Graphic			SOIL DESCRIPTION	REMARKS					
 - - /	-	/	6577	24	1	17.53		77	0-2	TO	P SOIL WJORGANIC	_ * AREITEARY _ SITE DATUM					
- 2 - 3	-	2	7 8 23	24			CL	70	2-4		RK CLAY TRACE SAND						
- 4	- 4		41				CL_	1//	4-6		LLOW CLAY TRACE						
- 5	-	3	20 25 23	24	6		CL	77		61	AUEL HARD	_					
	-6	4	18 22 25	<u>2</u> 4	12		•	(//	6-8		SHT BROWN TO ERAY AY VERY HARD						
 - & -	8		27 10 13			215	GL	1/2	8-10	· · · · · · · · · · · · · · · · · · ·	SAME						
- 4	<u> </u>	5	17	24	14	10 10	CL				37/95						
- "	-	6	14 13 24	24	14		CL	7	10-12	-	SAME						
	2 - 12		28 15 12				(L	4	12-14		SAME FRAY CLAY						
	-	7	15	24	12		CL										
			35						14-16		SAME GRAY CLAY						

SOIL BORING LOG (Continued)

Sheet $\frac{2}{}$ of $\frac{3}{}$

PROJECT: ARLINGTON HEIGHTS LANDFILL 20903

i		<u>-</u> -			· ·		1			CD= 21/4 CDC= 13	CONTINUE
	·					red	ery			Sampling Method(s): SFT 24" SPOON -	CO201/20008
7.87	Depth	Depth	٠ <u>٥</u> ٠	r E	riven	Length Recovered in/cm)	Graphic Recovery	Soil	Log	Sample Dimensions: 7710 Hammer Weight/Di	op: 140 1301N
7	Boring D (ft/m)	Sample [(ft/m)	Sample No.	Blows per 6 in/15 cr	Length Driven (in/cm)	Length R (in/cm)	phic	Unified Soil Classification	phic	Surface Conditions: GRASS	<u> </u>
; ;	Bor (ft/	San (ft/	San	Blo ni	Len (in/	Len (in/	Gra	- S	Gra	SOIL DESCRIPTION	REMARKS
. Date:	-	_ ,	8	15 20	24	8		CL		CAME	
	-/6	رما/ - ا		9		ļ	21		//	16-18]
	-/7	_	9	9 13	24	12				SAME	-
4	- -18	- -/8		21			نیرین	CL.			1
	-	-		359						18-20 SATME	
	-/9 -	- -	10	_	24	8	77		72	• • • • • • • • • • • • • • • • • • •	
Checked By:	-20	-20		13			26	CL	//	20-22	
hecke	- -2/	- ·]]	フラ	24	14	7	-	77	SAME]
0	-		<i>i</i> :	13.	,	' '		CL		<u>-</u>	-
	-22	-22 -		3	<u>-</u>		<u> </u>		22	22-24]
A	-23	-	12	8	24	16		CL		SANE	
7 7	-24	- -24		12				CL] .
5	- '	-	15	22	18	10	1 ₂ 7	6 .		24-25.5 CRAY CLAY LOWER PART YELLOW CLAY	-
By:	-25 -	 -25.5	/3	20	-	-		CL		W/ ERAUEL]
Logged By:	-26	-26		26	 					26-28 YELLOW CLAY W/ GRAVEL	67%
٦	27	_	14	17	24	18.	j_j^+			up to I"	6AS
	- -28	- -28		21				CL		-	7 27.55 (4-27-88)
	┝	-		12	, ,					28-29.5 YELLOW COARSE SAND LOWER FART GRAY COARSE	SATURATED
	-29	- -29,5	15	18	18	10		SP		SANZ	
U	-ઉ૦	-30		13						30-32 GRAY MEDIUM TO COARSE	SATURATEL
5	- -31	- -	16	19	24	20				SAND POORLY GRADED	
	-	- ,	10	21			1.	SP		. 	
. i.	-32 -	-32 -	· 	13			-			32-34	SATURATES
Drilling Contractor	-33	-	17	15 24	24	24		SP.		SAME	-
g Con	-34	- -34		24			*				ا المام الما
)rilling	F	- '		/3 /3						?4-36 -	CATURATED
	الواجه				Ц	<u> </u>	ادين		~		

SOIL BORING LOG (Continued)

PROJECT: ARLINGTON HT 16+FT C LANJEFILL 2090 B

MW-1 Boring No.: __

18 81	Depth.	Sample Depth (ft/m)	e No.	per 5 cm	Length Driven (in/cm)	Length Recovered (in/cm)	Graphic Recovery	Unified Soil Classification	ic Log	Sampling Method(s): SPT 24" SPON - C Sample Dimensions: 1,37/N Hammer Weight/Dr Surface Conditions: GRASS	t
7-6	Boring (ft/m)	Sample (ft/m)	Sample No.	Blows per 6 in/15 cm	Length (in/cm)	Length (in/cm)	Graphi	Unifie Classif	Graphic	SOIL DESCRIPTION	REMARKS
Date:	<u> </u>	-36	18	21 3 <u>2</u>	<u> </u>	24		SP		GARY MEDIUM to COARSE SAND POORLY BRADED	
	-37	_	P.	14 19	24	24		٤٤		36-38 SAME -	SATURATEL
MAK	-38 -39	- 1	20	16 16 27 27	Ø 14	24		SP		38-40 SA145	SATURATEL
J By:	-40	-40	20	27 35	27	29	(A))) F	, ,	_	
Checked By:	- - - -	- -								EOE 40FT 2" & PVC WELL INSTALLED.	
	<u>-</u>	1 1 1	,							SCREEN SET 38.5-28.5 FT	
J.J.D.	- - -	1 1 1								- - -	
Logged By:	- - -	-								- - -	
	_ _ _									- - -	
	- - -	1 1								- -	
78C	- - -	<u> </u>								<u>-</u> -	
tractor:	- - -	- - -						:		- - -	
Drilling Contractor:	- - -	- - -								- -	



MONITORING WELL MW-1 CONSTRUCTION SKETCH ARLINGTON HEIGHTS LANDFILL PROJECT SUBJECT 2090 B FILE NUMBER CHECKED WPIC 5-19-88 J.J.P COMPUTED DATE Steel casing protector w/cap & lock Ground surface concrete - 3,0 - 2" PVC Casing - Cement - bentonite grout --- Hollow stem auger boring -23.9 Pellets -28.0 -33.52 - Sand pack 2" 10 slot, PVC well screen 5ft long -38.52 Plug - 40,0

TESHARZA ENVIRONMENTAL SERVICES, INC.

A

SOIL BORING LOG

Sheet _____ of _______

PROJECT: ARLINGTON HEIGHTS LAISEFILL

2090 B

Location of Boring: NICHOLS EX 75' 2090GW-2	Water Depth: 5.3 5.77 Date: 4-27 5-/3 Time: 1020a 0300 b Hole Depth: 13.62 /3.5 Casing Depth:	Boring No.:	DENER
N F	Drilling Method (s): H. S. AUGER	Ground Elevation:	
		Casing Elevation:	90.41
Date 4-27-88 Time: 0830a	Hole/Casing Size (s): 7" BOR ING	Total Depth:	20.00
Date 4-27-88 Time: 10/5a	2" PVC CASING/SCREEN	Bottom Elevation: _	68.83

											<u> </u>			
1 600	pth	pth	·	_	iven	Length Recovered (in/cm)	Graphic Recovery	Unified Soil Classification	hic Log	Sampling Method (s): SPT 24" SPOON - CONTINUOUS Sample Dimensions: 1.37/N Hammer Weight/Drop: 140 150/N				
Date:	ng De	Sample Depth (ft/m)	Sample No.	Blows per 6 in/15 cm	# E	# E				Surface Conditions: CRASS				
	Boring (ft/m)		Samp		Length Driven (in/cm)	Leng (in/c	Grap		Graphic		SOIL DESCRIPTION REMARKS			
11/1	- · - /		/	6 15 14	24	14			7/	02	2 TOP (BIL ()LT W/GRAUEL AND _ * AREITMEY SAND LOWER 10" YELLOW CLAY SITE DATUM			
3y: _	- 2	-2		12		i	1	CL			WIGRAUEL			
Checked By	- 3	_	2	8 15	24	12			///	2-4	YELLOW CLAY W/GERUEL _			
1	- - 4	- -4		27			1.1	CL	1/2	2: 7				
	- - 5		3	8	24	2			·	4-6	SAME - MOIST			
	-6	-6		45				CL	ZZ	7.6	1			
ogged By	- - - 7	_	4	871	24	10				6-8	SILTY SAND WIGRAUEL SATURATED UP TO 1" YELLOW, SOME PEBBLES			
ا ، تـــــــــــــــــــــــــــــــــــ	_ _&	- -8		6		,	/// ///	SM			-			
	- S - - 9	ه ا ا	5	955	24	18				8-10	FINE TO MEDIUM W/GRAVEL			
	-,	<u> </u>		6			A COLOR	SP	-		UP TO 1/4"			
	_10 _ _ 11	_ 10 · _	,	7 5	011	0.11	-	- 0		10-12	2 SAME SAND MEDIUM SHTURATED TO COARSE			
	- 12		6	5 X X	24	24		SP						
ractor: _	-14 -13	- 12 - -	7	2 5 2	24	24	137	5P.	. 7	12-14	SAME UPPER FART SATURATED			
Drilling Contractor	- - 14	- - 14		/2 5				CL	<i>.</i>	14-16	CONTR B" GRAY CLAY MOIST			
Orillir -	_ _/ട			6			d1.1	C.L	1/1	7 / 0				

SOIL BORING LOG (Continued)

Sheet 2

PROJECT: ARAINGTON HEIGHTE LANDFILL 2090B

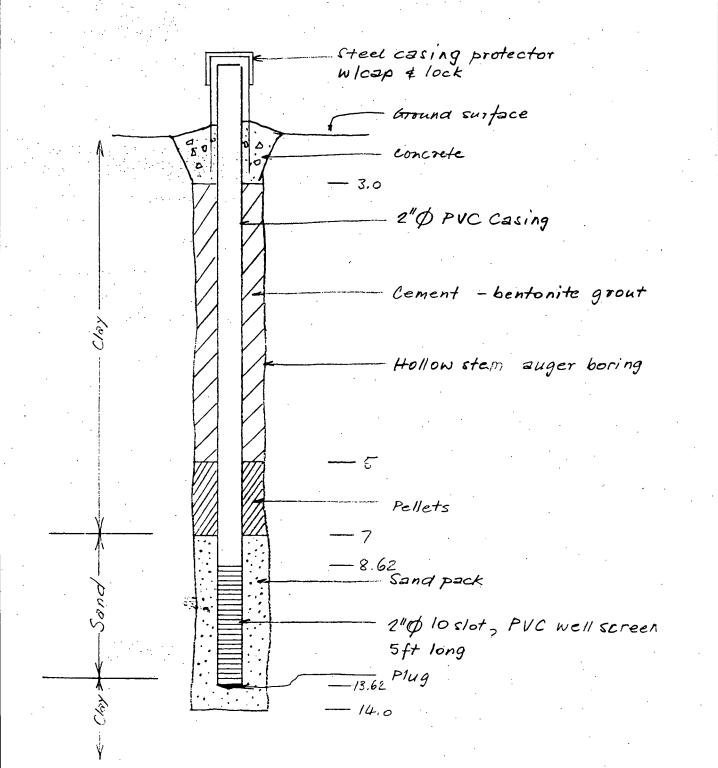
Boring No.: _____MW - 2

3 2	pth	apth			iven	Length Recovered (in/cm)	Recovery	lic ion	Log	Sampling Method(s): PT 24" SFOON Sample Dimensions: 1.3710 Hammer Weight/E					
2	g De	e De	Sample No	Blows per 6 in/15 cm	Length Driven (in/cm)	a) th	hic R	Unified Soil Classification	hic L	Surface Conditions: CRASS					
Date: 7-28-88	Boring Depth (ft/m)	Sample Depth (ft/m)	Samp	Blows per 6 in/15 cn	Length (in/cm)	Length (in/cm)	Graphic	Unif	Graphic	SOIL DESCRIPTION	REMARKS				
Date	_	_	8	10	24	24									
	-16	-16	-	57	<u> </u>				7	16-18 GRAY FLASTIC CLAY	J DRY				
	-/7	-	9	7 10	24	20	in	<u> </u>			- ;				
	-18	- - 18		/3					\mathbb{Z}	W 6	 				
	L.	F	,	10						18-20 SAME	DRY				
]	- 19°		10	10	24	/2		CL]				
Checked By:	-20	-20		24	·				//						
Chec	F	<u> </u>				,	:			EOB 20FT	-				
	_		,							2" & PVC WELL INSTALLED SCREEN SET 8.6-13.6 FT.					
	<u> </u>	-								SCREEN SET 8.6-13.6 FT.	-				
	<u> </u>	Ē													
\ >	_	L									_].				
>	-	F													
Logged By:	-														
Logg	-	_									4				
	- -	<u> </u>													
	F										<u> </u>				
	F.	Ē									_				
	<u>-</u> .	_									_				
)] -	F										_				
	<u> </u>	<u> </u>]				
 	L	L.									_				
Orilling Contractor:	F	_							·		-				
J Cont	L										-				
rillinç	F	-									-				



SUBJECT MONITORING WELL MW-2
CONSTRUCTION SKETCH
FILE NUMBER 2090 B

T. J. P CHECKED MK DATE 5-19-88 Page 1 of 1 Pages



MESHARZA ENVIRONMENTAL SERVICES INC.

SOIL BORING LOG

Sheet ______ of _____

PROJECT: ARLINGTON HEIGHTS LANDFILL

20903

ſ	Location of Boring:
	60'- 635'
	N PL

Drilling Method (s): #, S. AUGER

Boring No.: MEST SIDE

Coordinates: N_____

E______ Ground Elevation: 92.67*

Casing Elevation: 94.47*

	ء			<u> </u>	vered	overy					thod (s): SPT 24" SPOON - CO.			
Boring Depth (ft/m)	Sample Depth (ft/m)	Sample No.	Blows per 6 in/15 cm	Length Driven (in/cm)	Reco	Graphic Recovery	Unified Soil Classification	Graphic Log	Sample Dimensions: 1.37 /N Hammer Weight/Drop: 140 / 30					
					ength n/cm)				Surface Conditions: 6-RASS SOIL DESCRIPTION REMARKS					
<u>—</u>	8 -		3					-	0-2		TOP SOIL	* ARBITEARS		
- - /		,	5	24	8							SITE DATE		
_	<u></u>	,	7					7			LOWER 4" BROWN CLAY	_		
_ 2	- 2		3					12	2-4		DARK BROWN CLAY	=		
- - る		2	33	24	6]		
- .	 -	_	3	,		77	·					- V (1 25		
_4	-4		4			نند	! 	//	4-6			$-\frac{\sqrt{3.8}}{3.8}$ (4-25-		
- -5		3	57	24	6						SAME]		
	- ,		10			_						_		
_ 6	-6		7.	 		-		/	6-8		EKBROWN CLAY			
- - 7	_ '	4	7	24	8						WER THRT YELLOW	_		
- 6	_ &		12								7	-		
_ & _	— · X		I					7	8-10		LOW SANDY CLAY, TRACE	WET		
-9	-	5	455	24	8						ESAND SINGLE GRAIN FUEL UP TO 1/4"	- ·		
-	- - 10		7									┨		
_ <i> 0</i> -			6		-	1			10-12		Ph STNOY CLAY	WET		
<i>– 11</i>	-	6	6	24	8					1/2	SINGLE GRAVEL UPTO	-		
- -12	- 12		10								· 	- I		
- 1 L. -	_ /_ ,		10						12-14		LOW DARK GRAY SAND	SATURATE		
_/3	<u> </u>	7	11 13	24	8						BIVM , SOME GREAUEL	_		

HESHARZA HEIRONNENTAL SPRICES INC.

SOIL BORING LOG

Continued)

Sheet 2 of 2

PROJECT: ARLINGTON HEIGHTS LANGFILL 2090E

SPT 24 StOON CONTA DOUS Sampling Method(s): Length Driven
(in/cm)
Length Recovered
(in/cm) Graphic Recovery Sample Depth (ft/m) 1.37/N Hammer Weight/Drop: 140 / 30/N Boring Depth (ft/m) Unified Soil Classification Graphic Log Sample Dimensions: _ Blows per 6 in/15 cm Sample No. CRASS Surface Conditions: SOIL DESCRIPTION REMARKS MEDIUM GRAY SANIL 24 8 10 SP 16 - 16 16-18 SATURATED 5 SAME 8 24/10 12 S.P 18 18 - 18 DRY 4 GRAY 18-20 24/10 10 15 CL 22 -20 -20 10 20-22 SAME 12 24/12 21 16 CL 16 -22 -22 22.24 SATUT -23 24/10 12 12 A CLM 5 24 24 FT EOB Logged By: 2 & PVC WELL INSTALLED: SCREEN SET 10.7-15.7 FT. 5 Drilling Contractor:

ARLINGTON HEIGHTS LANDFILL MONITORING WELL MW-3 PROJECT 2090 B CONSTRUCTION SKETCH FILE NUMBER COMPUTED J. J. P 5-19-88 CHECKED MPK DATE . Pages Steel casing protector w/cap & lock Ground surface concrete 3.0 2" PVC Casing Cement - bentonite grout Hollow stem auger boring -8.0 Pellets 9.0 -10,7 ___ Sand pack 2" \$ 10 slot , PVC well screen 5ft long -15.7 Plug -24.0

HESHARZA ENVIRONMENTAL SERVICES, MIC

SOIL BORING LOG

Sheet / of 2

PROJECT : HELINGTON HEIGHTE LANDFILL

2090 B

Location of Bo	ring:	Ke	
NICHOLS		<u> </u>	
N V 8090	60°0 06W= 4	COTT Ave	

4-28-88 Time: 0900a

Water Depth: $\frac{17.7}{4-22}$ $\frac{17.45}{5-13}$ Date: $\frac{0330p}{81.90}$ $\frac{0/15p}{31.55}$ Hole Depth: $\frac{31.90}{31.55}$

Drilling Method (s): H.S. AUGER

Hole/Casing Size (s): 7" BORING

Boring No.: NW-4

Feature: NE COENER

Coordinates: N______

E_____ Ground Elevation: 99.62*

Casing Elevation: 102,52*

Total Depth: 34.0

Total Depth: 34.0

Bottom Elevation: 65.62

	Compl	Date _ eted:_	1-28	2-88	Tim	ne :	31	S _P	-			JC CASING/SCREEN Bottom Elevation: 65	
7-23-11						red	ery			Samp	ling Me	ethod (s): SPT 24" SPOON - CONTINUC	
7 :	epth	epth	ö	. E	Length Driven (in/cm)	Length Recovered (in/cm)	Recov	oil	Log	Sampl	le Dim		3010
Date:	ايا	Sample Depth (ft/m)	Sample No.	Blows per 6 in/15 cm	ag (E	gt (E	Graphic F	Unified Soil Classification	Graphic	Surfac	ce Cond	ditions: GRASS	
	Boring (ft/m)	Sam (ft/r	Sam	Blo oin	Len (in/	Len (in/	Gra	고 다음	Gra			SOIL DESCRIPTION REMA	RKS
MIL	_ _ 1].	/	669	24	8				0-2	i	P SOIL 2" - * ARBITE - SITE D	
Ву:	- -2	- -2	<u></u>	13	,			CL	\mathbb{Z}	21.	· •	LACK CLAY	
Checked By:	- 3	_	2	614	24	12			7/	2-4	<u>/</u> 50	LACK CLAY	
	- - 4	- -4		9				CL		4-6		PRI CLAY WITTERCE	
7	- -5	<u> </u>	3	69 10	24	//	:: :		77			SILT	
By:	-6	-6-		13 5 5				CL	1/2	6-8		EL CLAY	
Logged By:	_ _ フ	_ _	4	5	24	12		CL			10	WER PART BROWN CLAY	
	_ & 	-B		10					1/2	8-10		HT EROWN CLAY TRACE	
	_ 9 _	_	5.	9 14 14	24	14		CL			-	ILT AND VERY FINE AND GRAVEL UPTO 1/4"	
ار	_ 10 _	- lo : -		8 9					//	10-12		SAME	
1	// ·	-	6	12	24	l .	1000	CL					
ctor:	-12 - -13	/2 - 	7	11 18 23	24	18			7	12-14	RO	UNLED GRAVEL UF TO	
Drilling Contractor	_/S _ _/4	- - /u		34	-7			CL		-1, 12	1/2		
rillin	L',	_		10 14				CL		14-16		_	

SOIL BORING LOG (Continued)

Sheet $\frac{2}{}$ of $\frac{2}{}$

PROJECT: ARLINGTON HEIGHTS LANDFILL 2090B

Boring No.: NIW-4

Sample Dimensions: 7.37/1/2 Hammer Weight/Drop: 140 Soll Description Rema Settle Dimensions: 7.37/1/2 Hammer Weight/Drop: 140 Soll Description Rema Settle Dimensions: 7.37/1/2 Hammer Weight/Drop: 140 Soll Description Rema Settle Dimensions: 7.37/1/2 Hammer Weight/Drop: 140 Soll Description Rema Settle Dimensions: 7.37/1/2 Hammer Weight/Drop: 140 Soll Description Rema Settle Dimensions: 7.37/1/2 Hammer Weight/Drop: 140 Soll Description Rema Settle Dimensions: 7.37/1/2 Hammer Weight/Drop: 140 Soll Description Rema Settle Dimensions: 7.37/1/2 Hammer Weight/Drop: 140 Soll Description Rema Settle Dimensions: 7.37/1/2 Hammer Weight/Drop: 140 Settle Dimensions: 7.37/1/2 Hammer Weight/Drop: 140 Soll Description Rema Settle Dimensions: 7.37/1/2 Hammer Weight/Drop: 140 Settle Dimensions: 7.37/1/2 Hamme	· · · · · · · · · · · · · · · · · · ·											
20 22 SAME 21	CONTINUOUS	Sampling Method(s): SPT 24" SPOON (ery	red						
20 22 SAME 21	t/Drop: 140 / 30 /N		ارة ق	Soil	Recov	ecove	riven	LE	<u>0</u>	epth	epth	1/2
20 22 SAME 21		Surface Conditions: GRASS		fied S sifica	hic	th (m	th D	vs per	ple N	ple [g c	127
-16 - 16 - 16 - 16 - 17 - 9 19	REMARKS	SOIL DESCRIPTION	Gra	Clas	Gra	(in/o	Lenç (in/c	Blov 6 in	Sam	Sam (ft/n	Bori (ft/n	
10 18 18 19 24 22 CL 17		GRAY CLAY, LOWER PART		CL	 /!!	20	24	22	χ		_	Date
18	· :	16-18 GRAY CLAY TRACE	<u> </u>							-16	-16	
18		0 = 2) LT		CI		99	24	14	a	_	17	
10	- V (4-28-88)				111111	126			7	_	-	
21 - 11 /2 24 20 CL SAME 22 - 22 - 22 - 22 - 22 - 22 - 22 - 22	77	·			14	 -				- /8	1/2	1
21 - 11 /2 24 20 CL SAME 22 - 22 - 22 - 22 - 22 - 22 - 22 - 22]	SAME		CL		20	24	9	10		19	
22 - 22 22 22 22 22 22 22	4.				1975 1975 1975			1	, -	_	-	! <u>خ</u>
22 -22 22 27 24 20 CL 22-24 SAME 24 24 6 24 22 CL SAME 24 25 25 26 26 26 26 26 26					11	 -		9.		-20	-20	g pax
22 - 22 22 22 22 22 22 22	_	SAME		CL	La	20	24	12	11	_	21	Check
22 22 7 24 29 29 20 CL 22-24 SAME 24 24 24 18 24 22 CL 24-26 SAME 25 13 14 24 22 CL 26-28 SAME 26 26 26 6 7 7 26-28 SAME 27 26-28 SAME 28 28 11 18 18 CL 26-29.5 GRAY QAY 29 - 15 16 18 18 CL 20WER 2" SILT WET LOWER 2" SILT 30-32 GRAY POORLY GRADED SATURATION SATURATION SATURATION SATURATION SATURATION CL 23-24 GF AV CLAY DRY	-	· · · · ·		·	Shie	:	,	22	.,	_	}	I
23 - 12 14 24 20 CL 24 - 24		•						7		-22 L	-22	
24 - 24 18 24 22 CL 24 - 26 57ME 25 - 13 14 24 22 CL 57ME 26 - 26 26 26 26 27 27 - 14 12 24 21 CL 57ME 26 - 29 5 GRAY QAY WET 20 - 29 5 15 16 18 18 CL 10 WER 2" SILT 29 - 29 5 15 16 18 18 CL 10 WER 2" SILT 27 29 5	4	SAME		CL		20	24	9	12	-	-23	
25 - 13 14 24 22 CL SATME 26 - 26 - 26 - 16 19 24 21 CL SATME -27 - 14 12 24 21 CL SATME -28 - 28 11 18 18 CL LOWER 2" SILT -29 - 15 16 18 18 CL LOWER 2" SILT -30 - 30 - 30 7 SATME -31 - 16 15 24 24 SW CL LOWER 6" CLAY -32 - 32 - 32 - 32 - 32 - 32 - 32 - 32	-	· · · · · · · · · · · · · · · · · · ·			١,		,	1 / -	. –	-	<u> </u>	2
26 - 26 13 14 24 22 CL						 		.6		-24	-24	Ь
26 - 26 19 24 21 CL SAME -27 - 14 12 24 21 CL SAME -28 - 28 11 18 18 CL LOWER 2" SILT -29 - 15 16 18 18 CL SAMA -21 - 16 15 24 24 SW SAND LOWER 6" CLAY DRY DRY		5#7NE		CL		22	24	14	13	_	-25	1
28 -28 11 26-29.5 GRAY QAY WET -29 - 15 16 18 18 CL LOWER 2" SILT -29 - 30 - 30 7 8 24 24 SW SAND LOWER 6" QAY -21 - 16 15 24 24 SW CL LOWER 6" QAY -22 - 32 32 32 32 32 GRAY POORLY GRADED SATURATED DRY	<u> </u>				7					20	-	d By
28 -28 11 26-29.5 GRAY QAY WET -29 - 15 16 18 18 CL LOWER 2" SILT -29 - 30 - 30 7 8 24 24 SW SAND LOWER 6" QAY -21 - 16 15 24 24 SW CL LOWER 6" QAY -22 - 32 32 32 32 32 GRAY POORLY GRADED SATURATED DRY			7					6		_20	-26	-odde
28 -28 13 26-29.5 GRAY QAY WET LOWER 2" SILT WET LOWER 2" SILT 30-32 GRAY POORLY GRADED SATURATED SATURATED SATURATED SATURATED LOWER 6" QLAY DRY	-	SATUE		CL		21	24	12	14	-	-27	
25 - 15 16 18 18 CL / LOWER 2" SILT 30-30 7								13		- -28	28	•
30-32 GRAY POORLY GRADED SATURATED S	- WET		77			10	10				-	
0 30 -30 7 8 30-32 GRAY POORLY GRADED SATURAL SAND LOWER 6" CL 7 DRY	-	WWER Z" SICI				/8	18		15	-	-29	
SAND 16 15 24 24 SW : SW : LOWER 6" QLAY 22 32 22 20 DRY				""								
16 15 24 24 SW .: LOWER 6" CLAY SZ 32 32 20 CL 7 SRY	SATURATED		′.	,				7				
32 32 23 CL DRY	-			SW		24	24	15	16	_	-31	
	DRY		\mathbb{Z}	CL	<u> </u>	ļ				32	182	
17 27 4 12 CL 7 1WOTALL		32-14 GKAY CLAY		-							-	tor:
5 F F 2c	2"Ø PUC WELL INSTALLÉD.	•	7/	21	-	12	24	21:	17	-	-33	ıtracı
SCREEN	SCREEN SET			<i>L L</i>	igspace			35		-24	34	g Cor
EOB 34 FT - 26.9-31.9	-26.9-31.9 FT.	EOB 84 FT								-)rillin



MONITORING WELL MW-4 ARLINGTON HEIGHTS LANDFILL PROJECT SUBJECT CONSTRUCTION SKETCH 2090 B FILE NUMBER 5-19-88 J.J.P CHECKED LAKE Page / of / Pages COMPUTED DATE Steel casing protector w/cap & lock Ground surface concrete - 3,0 2" PVC Casing Cement - bentonite grout - Hollow stem auger boring -23,0 Pellets - 25,0 -26.9 Sand pack 2" ploslot, PVC Well screen 5ft long ____31.9 Plug -34.0

SOIL BORING LOG

Sheet _____ of _____

PROJECT : ARLINGTON HEIGHTS LANDFILL

2090B

Location of Boring:	
K	
7920906W-S	
25'	
1210'	
N T	

5.1 Water Depth: _ 4:26 5-13 Time: 0430p 1040a

Hole Depth: 21.35

Casing Depth: Drilling Method (s): #, S, AuGER

Boring No.: MW-5

Feature: WEST SIDE

Coordinates: N___

Ground Elevation: ___

Casing Elevation: _

Date: Z-27-79 g Depth		Depth	No.	er cm	Oriven	Length Recovered (in/cm)	Recovery	Soil ation	Log				t/Drop: 140 / 30 1/3
Day Day	(ft/m)	Sample I (ft/m)	Sample No	Blows per 6 in/15 cm	Length Driven (in/cm)	Length F (in/cm)	Graphic	Unified Soil Classification	Graphic	Surfa	ce Con	SOIL DESCRIPTION	REMARKS
	/	- -	1	457		12			1/	02	,	TOP SOIL 3" YE LLOW CLAY	_*ARBITRARY _ SITE DATUM
cked	2	- - 2 -	2	10671	24	8		CL	//	2-4		YELLOW SANDY CLAY, W/BRAVEL UP TO !"	
-	4	- - 4		4777		-		CL	\mathbb{Z}	4-6		ERAYSAND 8"	MOIST
3 -	5	- - - 6	3	9	24	./0		CL		6- 8		EROWN CLAY	-
Logged By:	7	- - - - &	4	12 16	24	8		CL	<u>//</u>	4			.]
	9	 	5	8 10 11	24	11		CL		8-10		ERAY CLAY	
	10	10 	6	779	24	15	1	CL		10-12		SAME	
' I	12	- 12 -	7	14671	24	18		CL	7	12-14	·	SAME	DRY
Drilling Contractor:	14	- - 14 -		10 15 4 5				£K)		14-16			

SOIL BORING LOG

(Continued)

Sheet $\frac{2}{}$ of $\frac{2}{}$

Boring No.: _ MW-5

PROJECT: ARLINGTON HEIGHTS LANDFILL 2090 B

HESHARZA.

SAT24" SFOON - CONTINUOUS Sampling Method(s): Sample Depth (ft/m) Hammer Weight/Drop: 140 / 3010 Length Driven (in/cm) 1,3710 Sample Dimensions: _ Blows per 6 in/15 cm Sample No. Length F (in/cm) Surface Conditions: SOIL DESCRIPTION REMARKS GRAY FINE SILTY SATUS 似もて LOWER PART GRAY CLAY -16 16 16-18 GRAY CLAY W/FINE WET 8 GRAY SAND WELL GRADED 24 24 1 17 SP 34 -18 -18 18 18-19.5 FINE GRAY SAND SATURATED SP 16 WELL GRASED 18 17 19 10 29 -19.5 Checked By: 20-20 8 20-22 GRAY SAND 8 24 24 SP II17 LOWER PART SILTY CLAY DRY CL 18 22 -22 EOR 22 FT 2" & PUC WELL INSTALLED 5 SCREEN SET 16,5-21,5 FT. Drilling Contractor:



MONITORING WELL MW-5 ARLINGTON HEIGHTS LANDFILL PROJECT CONSTRUCTION SKETCH 2090 B FILE NUMBER 5-19-88 J.J.P CHECKED ML Page / of / Pages COMPUTED DATE Steel casing protector W/cap & lock Ground surface - concrete - 3,0 - 2" PVC Casing Cement - bentonite grout Hollow stem auger boring -12.0 Pellets - 150 -16.5 Sand pack 2" ploslot, PVC Well screen 5ft long 22,0

SOIL BORING LOG

2090B

PROJECT: APLINGTON HE IGHTS LANDFILL

Location of Boring: 209000-60 460

38.5 Water Depth: 5-13 Time: 0930a 0830a Hole Depth: Casing Depth:

Drilling Method (s): HS Aubtr

Hole/Caşing Size (s): _

Boring No : MW-6 Feature: EAST SIDE

Coordinates: N

Ground Elevation: 120.15 123.15

Casing Elevation: _ Total Depth: __

Boring Depth (ft/m)	e Depth	No.	per 5 cm	Length Driven (in/cm)	Length Recovered (in/cm)	Graphic Recovery	Unified Soil Classification	c Log	Sampling Method (s): SPT 24" SPOO Sample Dimensions: 1.37 M Hammer Wei	ght/Drop: 140_, 301
Boring (ft/m)	Sample (ft/m)	Sample No.	Blows per 6 in/15 cm	Length (in/cm)	Length (in/cm	Graphi	Unified Classif	Graphic	SOIL DESCRIPTION	REMARKS
_ }	_ _	,	466	24	8	400)-2 TOP SOIL	- X AREITRARY - SITE DATUM
-2	- - 2 -	· 	9572	<i>C</i> :	_	1122			2-4 DARK CLAY WIMEDIUMST AND ERAVEL	
-3 - -4	- - - 4	2	12 15 9	24	9	A .	CL		4-6 DARK CLAY	
- -5 -	 	3	5	24	10	1. O	CL	7		
-6 - -7	- 6 - -	4	993	24	In				5-8 YELLOW CLAY	-
- - & -	- - 8 -		75				CL		8-10	
- 9 - - 10	- - - 10	5	11 21 15	24	12		CL		SAME	-
- ,	- // - -//	6	15 30	12	3	88	CL	7	WITH BOULDER	
- - 12 -	- -12 -		32 &	2					12-14 DARK CLAY	
_18 - -14	- - -14	7	11 14 23 8	24	6	£	CL	Z	14-16	

SOIL BORING LOG (Continued)

Sheet _____ of ____3

HES HARZA ENVIRONMENTAL SERVICES, INC. PROJECT: ARLINGTON HEIGHTS LANDFILL 2000 B

						þ	Υ.			Sampling Method(s): SAT 24" SADD O	DNT/NUOUS
18-	Depth	Depth	Ċ	_	iven	Length Recovered (in/cm)	ecover	ie ie	og	Sample Dimensions: バスブル Hammer Weight/Di	op: <u>140 30 //J</u>
88-27-6	1 _	Sample Do (ft/m)	Sample No.	Blows per 6 in/15 cm	Length Driv in/cm)	gth Re cm)	Graphic Reco	Unified Soil Classification	aphic L	Surface Conditions: GRASS	
	Boring (ft/m)	Sarr (ft/r	Sarr	Blo in	Length (in/cm)	Length (in/cm)	Gra	C G	Gra	SOIL DESCRIPTION	REMARKS
_ Date:	L , ,		8	12	24	12		CL		GRAY CLAY	
	-16 -	- /6 -		109					X.Z.	16-18 SAME]
	-17	_	9	14	24	12	3 5.	CL	7		
N. C.	-18	- 18	· .	29 12				C 6	4		
	-19	- ·	10	17 20	24	0				NO SHMPLE	
By: _	-20	- -20		21						-	
Checked By:	F	-	.,	15	18	16		CL		20-21,5 GRAY CLAY	
ਰੰ ।	-2/ -		_// 	19		<u> </u>		LL	4		
	-22	-22		8					 .	22-23,5 GRAY CLAY, SOME	
	-23	_	12	9 13	18	10		CL		COARSE SAND AND GRAVEL UPTO 1/2"	
A	-24	- -24								24-26 UPPER PART (IFT) GRAY	
6	- -25	_	13	7	24	18	5	SM		SILTY SAND	
J By:	├ .	-	<i>\(\)</i>	10 13	21			CL		LOWER PART GRAY CLAY	-
Logged By:	-2.6 -	-26		7			11:2		24	26-28 CRAY CLAY	10% GAS
-	27	-	14	12	24				//	-	
	-28	-28		/& 8				CL	4	28-29.5	1
	-29	- -	15	9	18	12		CL		SAME	2,5% C1 S.
	-30	- -30		12			#! <u>*</u> 1				.
U	-	-	17	611			T.			30-32 SAME	17% 645
75	-?1 -	- -	16	14	24	18		CL		-]
<u>;</u>	-32	-32 -		7						32-34.	6-0/ ()
Drilling Contractor:	_33	- .	17	93	24	18		CL		SAME	23% 645
ng Con	- -34	- -34	-	14			*		4	34-35,5 CRAYCLAY	1
Drillir	-	-	18	12	18	6				U 1 3030	23% GAS

SOIL BORING LOG (Continued)

Sheet 3 of 3Boring No.: MW-6

PROJECT: ARLINGTON HEIGHTS LANDFILL 20903

HES HARZA ENVIRONMENTAL SERVICES, INC.

	_								•		
4					_	ered	very			Sampling Wethod(s):	W 5
22-8	Depth	Depth	No.	er cm	Length Driven (in/cm)	Length Recovered (in/cm)	Recov	Unified Soil Classification	Log	Sample Dimensions: 1.37 M Hammer Weight/D	rop: 140 / 30 /N
7-7	Boring (ft/m)	Sample (ft/m)	Sample No	Blows per 6 in/15 cr	Length (in/cm)	ogth (cm)	Graphic	Unified Classific	Graphic	Surface Conditions:	
	Boı (ft/	Sar (ft/	Sar	Blo 6 ir	(in	ii Je	er5	5 రొ	ű	SOIL DESCRIPTION	REMARKS
Date:		- <i>35.</i> 5	·	37				CL	\mathbb{Z}		
	-36	-36		24			! · . ī			36-37.5 MEDIVM TO COARSE	21% 645
	-37	-	19	53	18	16		sp	<i>v</i> .	SAND W/ROULDERS	
		_		54	·	ļ	1		- T.		_
1/1	-3§	-38		19			L.	SP		38-40 GRAY SAND MEDIUM TO	-
11.	- -39	_	20	19	211	24				COARSE POORLY GRADED	22% 645.
	Γ	_	20	27	7	27	1	Sβ		SOME SILT IN MILLER	$\frac{\nabla}{39.5}$ (5-3-88)
Checked By:	40	-40		1)	ļ <u></u>		<i>I</i> /2	<i>≥ r</i>	<u></u>	40-42 GRAY STAND MEDIUM TO	SATURATED
ecke	L ₄₁	_	21	19	24	10	V.			COARSE WITH GRAVEL	22% 645
5		_	21	1 ''	21	1/8		SP	7:	Uf to 1/2"]
•	-42	-42		18			11		27.	42-44 GRAY COARSE SANI	SATURATED
	- -43	-	22	1	24	24		SΡ		AND GRAVEZ	21% GAS
a .	F-7-3	_	22	′′	64	24]
1>	44	-44		17		· 			- '	44-46	SATURATED
5	- -45	-	00	۔ ا	24	24	9 - 1 - 4 1 - 4 1 - 4 1 - 4 1 - 4 1 - 4	CD	,	SAME	
By:_	L-13	_	23	19	27	24		SP	^ -	·]
Logged By:	46	-46					, i		· .		
Log		-								EOR 46FT]
		_									
	⊢ '	- '								2" & PVC WELL INSTALLER SCREEN SET 38.8-43.8FT.	†
	-									SCREEN 36) 30.0 13.0 1	<u> </u>
	F	_								<u>-</u>	1
750	-	_								-	
7	-	-								-]
		<u>-</u> ,								-] '
	-	-								-	-
ctor:	- .	_	!							-]
ontra		_								- -]
Orilling Contractor:	-	-								-	<u> </u>
Orillir	!	<u>-</u>									1



ARLINGTON HEIGHTS LANDFILL MONITORING WELL MW-6 PROJECT SUBJECT CONST RUCTION 2090 B SKETCH FILE NUMBER 5-19-88 J.J.P igstyle of $oldsymbol{igstyle}$ COMPUTED DATE ∠_ Pages CHECKED Steel casing protector w/cap & lock Ground surface concrete 3,0 - 2" PVC casing Cement - bentonite grout Hollow stem auger boring - 28.0 -- Pellets —38.8 ——Sand pack . 2" \$ 10 slot, PVC well screen 5ft long - 46,0

MESHARZA

SOIL BORING LOG

113.15

PROJECT: ARLINGTON HEIGHTS LANDFILL

20905

SOUTH SIDE

Boring No.: NW-7

Coordinates: N

Loca	tion of Boring:	7
	PL	
W	20906W-7 275'	

Water Depth: 32 5 32 5

Date: 5-6 5-13

Time: 0430p 0730a

Hole Depth: 47.55 47.5

Casing Depth:

Ground Elevation:

Drilling Method (s): # S. AUGER

Casing Elevation: 115.40 ± 3 Total Depth: 48.0

Hole/Casing Size (s): 7" BOR ING.
2" PVC CASING/SCREEN

Bottom Elevation: 65,15

Date Started: 5-6-88 Time: 0815a Completed: 5-6-88 Time: 042op

A

S

Date: 7-28-88	Boring Depth (ft/m) Sample Depth (ft/m) Sample No. Blows per 6 in/15 cm Length Driven (in/cm) Length Recovered (in/cm) Graphic Recovery Unified Soil							Unified Soil Classification	c Log	Sampling Method (s): SPT 24" SPOON - CONTINUOUS Sample Dimensions: 1/27/N Hammer Weight/Drop: 140 / 30/N Surface Conditions: GRASS					
_ D	oring t/m)	ample t/m)	ample	lows i in/15	ength n/cm)	ength n/cm	iraphi	Inifiec Lassifi	Graphic	Surface Conditions.	DEMARKS				
$ _{\mathcal{L}}$	8 ±	ö±	Š	89	€ ت	≎ تــ	g) 	9	SOIL DESCRIPTION 0-2 10A SOIL 411	REMARKS				
PPE	- , - ,	- -	1.	6	24	10	# C	CL	7/	6" BROWN CLAY	* ARBITRARY SITE DATUM				
J By:	-2	- 2	-	5					4	2-4 BROWN CLAY WISILT					
Checked By:	- - 3	- -	2	13 12	24	20		CL		AND SOME GRAVEL					
1	_4	- _ 4		23			1		4	// H181 B201111 0/11					
4 6	- 2 -	_	3	57 R	24	12	100	0.1	77	4-6 HARD BROWN CLAY	-				
. JJ	-6	- - 6		/3	·		压的	CL	\mathbb{Z}		· ·				
Logged By:	- - 7	- ·	4	11 14 21	24	16		CL.	1/	6-8 SHME	-				
ا بـ	- 8	- - &		29			116		\mathbb{Z}						
		_		9						8-10 HARD ERAY CLAY					
	_ <i>9</i> 	 -	5	16 27	24	15		CL			-				
	-10	-/0		7			11:3		14	10-12	· · · · · ·				
)	_//	_ _ _	6	1015	24	14		CL		SAME					
	-12	- 12		17			12		4	12-14					
Drilling Contractor:	- -13	- -	7	10	24	10		ĊL	77	CHAE	<u></u>				
3 Con	 /4.	- - 14		12:				CL							
rillinç	-	_ ′		11				_							

SOIL BORING LOG (Continued)

Sheet _

PROJECT: AFILINGTON HE164TS LANDFILL 2090 E

Boring No.: _

	·					pa	гy			Sampling Method(s): (FT 24' SFOOA)	
44-82-	Depth	epth	0.	LΕ	riven	Length Recovered (in/cm)	Secove	ioil	Log	Sample Dimensions: 1.371/J Hammer Weight/D	rop: <u>/40</u> / <u>30ル</u>
12-2	Boring Do (ft/m)	Sample Depth (ft/m)	Sample No.	Blows per 6 in/15 cm	Length Driv (in/cm)	Length R (in/cm)	Graphic Recov	Unified Soil Classification	Graphic I	Surface Conditions:	
Date	Bor (ft/	San (ft/	San		Len (in/	Len (in/	Gra	<u>້</u> 5 ຮື	Gra	SOIL DESCRIPTION	REMARKS
Da	- -16	- -16	8	13	24	//		CL		GRATCLAY	.
	<u> </u>	- / 0		11	18	6				16-17.5 SAME] .
\searrow	- <i>17</i>	- -/7.5	9	//				CL	\mathbb{Z}	-	·
9	-18	- 18 -	-	5 &						18-20 CRAY SILT	MOIST
	-19	_	10	8	24	18	(C)			-	.
1 By:	- -20	- -20		10	<u> </u>			ML		- 20-21.5	WET
Checked By	- -21	- -	// .	9	18	18		ML		1 AME	102/
5 	-	-21.5 20	11	6	<u></u>						4.
	-22 -	-22 -		457		,				72-24 GRAY SILTY CLAY W/GRAUEL UPTO 1/2"	DRY
4	-23 -	 -	12		24	19		CL		-	
7	-24	-24		10					$\frac{1}{2}$	24-25.5	DEY
D	-25	-	13	8	18	18	¥	CL		SAME	
Logged By:	-26	-25 -26		7					7	26-27.5 GRAY SILTY CLAY	149 B
Logic	- -27	-	14	8	18	18	7.	CL		26-27.5 GRAY SILTY CLAY	
	-	-27 <u>.5</u>	<i>1</i>						4		-
	- <i>28</i> -	-28 -		5			7 .5		7,	28-30 CAME]
	-29	_	15	8	24	19	10.	CL		-].
	-30	-30		7					4	30-315	
780	-31	_	16	10	18	15		CL		SAME]
	- -32	-315 -32				· · ·	10		-		
ctor:	- -33	_	17	フリ	18	18		cL		32-33.5 (AMTE DI/SOME ETALLES	$\frac{\nabla}{32.5}$ (5-6-88)
Sontra	-	– –ਡੂੰ∑ਾ		12			77.6.2		9/		
Drilling Contractor:	-34 -	-34 -		8/0]
Ö	2-			10							

PROJECT: ARLINGTON HEIGHTS LANDFILL 2090 B Boring No.:

						ed	7			Sampling Method(s): SPT 241 SPOON CO	177NU0US
725-81	Depth	Sample Depth (ft/m)	<u>.</u>	l. Es	riven	Length Recovered (in/cm)	Recovery	Soil	Log	Sample Dimensions: // 37 / Hammer Weight/Drop	: <u>140 / 30 N</u>
20	ng D	iple [Sample No	Blows per 6 in/15 cm	Length Driv (in/cm)	gth R :m)		Unified Soil Classification	Graphic	Surface Conditions:	
	Boring (ft/m)	Sam (ft/r	Sam	Blov 6 in	Length (in/cm)	Length (in/cm)	Gra	C Clas	Gra	SOIL DESCRIPTION	REMARKS
Date:			18	// /2	24	8:	<u></u>	CL	7	GRAY SILTY CLAY	
	-36	-36		4					//	36 - 32	.
·	-37		19	4 10	24	22		CĿ.		SAME	
1	- -38	-38°	,	13							
S)	F 30	- 4 0		45			¥.			SZ-40 SAME	
	-39	- .	20	10	24	22		CL		-	
J By:	40	- -40		14					/	40-42 ERAY CLAY	
Checked By:	- -47	-	21	6 5 12	24	00		ĊL		40-42 ERAY CLAY	ŀ
ပ	F ⁴ /	1	21	12 14	21	22	<u>1</u> 2.2	CL			
	-42	-42		3					//	42-44 GRAY CLAY	DRY
	- -43		22	2	24	22		CL]	·
A	_ _44	_ ///		58 21			7 2 3	ML	\parallel	LOWER G" GRAY SILT	40157
77	-44	- 		10			.5			44-46 GRAY MEDIUM TO COARSE !	ATURATED
ļ	_45	_	23	13	24	24		SP		-	
Logged Ây:	-46	-46.		15	<u> </u>					46-47.5 GRAYMEDIUM TO COARCE	
Logi	47	,	QU	11	18	18		5 P	•	SAAJA	
j.	L . ,	-4/,3	24	21		_	Ţ.	_ `		LOWER 6" GRAYCLAY	ZO/ST
	-48	<u>-48</u>		<u> </u>							
	F	_	·							EOB 48FT	·
;	├ .	-								2" & PVC WELL INSTALLED _ SCREEN SET 42.6-47.6FT _	,
U	F	_									
27	-	_								-	
'											
ctor:	-	-									
ontra(F	<u> </u>									
Drilling Contractor:	-	<u>_</u>								-	
Drilli				·						<u> </u>	



ARLINGTON HEIGHTS LANDFILL MONITORING WELL MW-7 PROJECT CONSTRUCTION 2090 B SKETCH FILE NUMBER 5-19-88 J.J.P CHECKED INIC COMPUTED DATE Steel casing protector wlcap & lock Ground surface concrete - 3,0 2" PVC Casing Cement - bentonite grout Hollow stem auger boring - 26,0 -- Pellets 32,0 -42.55 — Sand pack 2" 10 slot, PVC well screen 5ft long 47.55 Plug - 42.0

SOIL BORING LOG



Sheet _____ of ____2__

PROJECT: ARLINGTON HEIGHTS LANDFILL

2090B

Locat	ion of Borin	g:	
NIC	HOLS Rd	100	/
	580'	-0	
Æ			A
'			

Ground Elevation: 98.65 *

Casing Elevation: 101.1 *

Casing Elevation: 707.7

Total Depth: 24.6

Bottom Elevation: 74.65

ole/Casing Size (s): 7" BORING.

	Comp												
70.07						pa	<u>7</u>			Samp	ling Me	thod (s): SPT24" SPOON COM	SUOUNTE
	Depth	Depth	Ġ	_	iven	cover	ecove	io ion	Log	Samp	le Dim	ensions: 13710 Hammer Weight/Dre	op: 140 /301N
Date:	g (c	ple Dر (ر	Sample No.	Blows per 6 in/15 cm	Length Driven (in/cm)	Length Recovered (in/cm)	Graphic Recovery	Unified Soil Classification	hic L	Surfa	ce Cond	ditions: GRASS	
	Boring (ft/m)	Sample (ft/m)	Sam	Blov 6 in	Leng (in/o	Leng (in/o	Gra	Clas Clas	Graphic			SOIL DESCRIPTION	REMARKS
4				4						0-2		TOP SOIL 3"	* ARBITRARY
	- 1	<u> </u>	1	4	24	7					4 11	CLAY W/SAND AND SILT -	SITE PATUM
ا <u>خ</u>	-			ų.				CL			7	_	·
Checked By:	-2	-2	٠.	5						2-4	BE	WWW CLAY W/GRAVEL	
heck	-3	_	2	6	24	12	370		7		UF	TO 1/4"	
O	F ,	├ .		13			1	CL		:		. -	
	-4	-4	-	5						4-6			MOIST
	-5	F I	3	5	24	12	.,		-		·	SAME	
)	<u> </u>	├ ,		45		ļ	3.	CL				-	
By:	 -6	-6		2			15.0	-	//	6-8			WET
Logged By:			4	2	24	12	197		77			SAME -	
۲	-	-		4				CL				· -	
	<u> </u>	- 8		4					M	8-10			·
			5.	6	24	11			—			SAME	MOIST
ı	F	-		10				CL				<u> </u>	
	-10	-10	·	12			7 ₩2.		4	10-12	BZ	COWN SANDY SILT	WET
)	_ _{//}	<u> </u>	6	25	24	10						FINE SAND AND	
	F .	_		28 22	- 1	, ,		Shir ML	li.		40	RAVEL UP TO 1/4"	
	-12	-12		10	·		Ċ I	/// L	: 1.5	12-14	UPP	FR PART (1 FT) LIGHT SATIL	SATURATED
3ctor	- -/3		ブ	//_	24	/3					F/N	E TO MEDIUM W/BROWN -	
ontra	L .		/	13	61	/3		5 P				E SAND AND GRAVEL UP TO 1/2 " = R PART COARSE SAND W/BROWN SPOT	
illing Contractor	-14	-14		<i>15</i>			116			14-16		A TITLE COMESE STIVL PIZEOWN SHOP	CATURATED
É	<u> </u>	⊢ 		20								·	

SOIL BORING LOG (Continued)

Sheet 2 of 2

PROJECT: ARLINGTON HEIGHTS LANDFILL 2090 B

Boring No.: MW-8

18-81	g Depth)	Sample Depth (ft/m)	Sample No.	Blows per 6 in/15 cm	Length Driven (in/cm)	Length Recovered (in/cm)	Graphic Recovery	Unified Soil Classification	hic Log	Sampling Method(s): SPT 24" SPCO (1) Sample Dimensions: 1,3710 Hammer Weight/D Surface Conditions: SRASS	.*
7	Boring (ft/m)	Samp (ft/m	Samp	Blow: 6 in/	Lengi (in/cr	Length (in/cm)	Grapl	Unifi Class	Graphic	SOIL DESCRIPTION	REMARKS
_ Date:	- - 16	- - 16	8	19 18		12		CL		BROWN CLAY W/BOULDER	DRY V (5-4-88)
7	-17 -17 -	-	9.	67911	24	22		5 P		16-18 BROWN AND BLACK SAND MEDIUM TO COHRSE POORLY GRADEL	16.0 SATURATED
3y:	- -19 -	- - -19.5	/o	8 10 22	18	16	おいる事	SP	1.7.5.7.1	18-19.5 WHITE COARSE SANDE AND GRAVEL UP TO 44	SATURATED
Checked By:	 -2/ -	-20 - -	11	8 14 18 24	24	24		sf		20-22 SATHE	SATURATED
	-22 -23 -24	-22 - - - -24	12	29212126	24	24		SP		22-24 SAME	SATURATED
	-27 - -		-							EOB 24 FT	
Logged By:	.	- -								2" & PUC WELL INSTALLED . SCREEN SET 17.4-22.4 FT.	- - -
	1 1 1	- -]
			-								
r:		_			·			-			
Drilling Contractor:	1 1 1	-									<u> </u>



MONITORING WELL MW-8 ARLINGTON HEIGHTS LANDFILL PROJECT SUBJECT CONSTRUCTION SKETCH 2090 B FILE NUMBER 5-19-88 J.J.P. CHECKED MPL COMPUTED -DATE Steel casing protector WICOP & lock Ground surface concrete 3,0 2" PVC Casing Cement - bentonite grout Hollow stem auger boring - 11,0 ----Pellets -15,0 -17.35 ____Sand pack 2" ploslot, PVC well screen 5ft long 24.0

Appendix B

Laboratory Soils Test Reports



TESTING SERVICE CORPORATION

457 EAST GUNDERSEN DR. • CAROL STREAM, ILLINOIS 60188-2492

June 27, 1988

HARZA ENVIRONMENTAL SERVICES, INC. 150 South Wacker Drive Chicago, Illinois 60606-4288

Attention: Mr. Farrukh M. Mazher

Dr. Lawrence A. DuBose, P.E., S.E. President Charles C. Luther, P.E. Executive Vice-President Bruce R. Poynor Engineering Geologist, V.P.

Charles R. DuBose, P.E., V.P.
Thomas J. Morris, P.E., V.P.

Scott B. Stueland, P.E.

Allen L. DuBose Director of Marketing Telephone 312 653-3920

Received 6/30/28

RE: L - 25,441

SOIL TESTING PROGRAM

ARLINGTON HEIGHTS LANDFILL

2090B

Dear Mr. Mazher:

We have completed laboratory testing on the above mentioned project. This information is on the enclosed Soil Test Data sheets. The water contents of alternate samples of cohesive material in the monitoring well bore holes is included in the following table.

BORING	SAMPLE	WATER CONTENT	BORING	SAMPLE	WATER CONTENT
1	1	33.9	5	7	14.0
1	3	19.0	5	9	17.9
1	5	15.9	5	11	15.6
1	7	15.6	6	1	25.0
1	9	12.0	6	3	22.6
1	11	14.0	6	5	16.5
1	13	14.0 11.5	6	7	14.6
2	1	16.5	6	9	14.9
2	3	21.6	. 6	11	15.4
2	7	16.7	6	13	14.5
2	9	18.1	6	14	13.8
3	1	20.9	6 .	15	14.9
3 .	3	22.7	. 6	17	13.4
3	7	12.3	7	1	26.6
3	9	15.2 12.4	7	.3	19.0
3	10	12.4	7	5	18.7
3	12	16.1	7 .	7	16.2
4	11	20.8	7	9	17.6
4	3	33.9	7	12	14.4
4	5	15.0	7	14	9.8
4	7	13.9	7	15	17.8
4	9	12.7	7	16	15.5
4	11	15.2	.7	18	15.2
4	13	13.4	7	19	15.9
4	15	23.9	7	21	14.3
4	17	15.1	7	23	13.7
5	11	13.3	8	1	21.8
5	3	15.9	8	3	17.3
5	5	14.5	8	5	17.5
5	6	15.0	8	8 11	12.4
			8	11	13.1

It has been a pleasure to assist you with this work. Please call if there are questions or if we can be of further service.

Respectfully submitted,

TESTING SERVICE CORPORATION

Lawrence A. DuBose

President

LAD:BW:tmc

Enclosures

Prepared By:

Buan Walker

Brian Walker Geologist

CLIENT: HA	RZA ENVIRONMENTAL SERVICES	 .	JOB NUMBER:_	25,441	_
			DATE:	JUNE 27, 1988	
		<u>.</u>			
PROJECT: _	ARLINGTON HEIGHTS LANDFILL		<u> </u>		

			·	· · · · · · · · · · · · · · · · · · ·	
LAB NUMBER	·]
BORING NUMBER	SB-1	\$B-2	SB-2	\$B-2	SB-3
SAMPLE NUMBER	1	1 TOP	1 MIDDLE	1 BOTTOM	2
	•				
DEPTH IN FEET	1-2.5	1-2.5	1-2.5	1-2.5	3.5-5
HRB CLASSIFICATION & GROUP INDEX	· OL	ОН	OL.	CL	CL
GRAIN SIZE CLASSIFICATION		-			
GRADATIONPASSING 1" SIEVE %	·				
GRADATIONPASSING 3/4" SIEVE %					
GRADATIONPASSING 3/8" SIEVE %	100			·	100
GRADATIONPASSING NO.4 SIEVE %	98		100	100	99
GRADATIONPASSING NO.10 SIEVE %	95		98 -	98	.98
GRADATIONPASSING NO.40 SIEVE %	89		93	83	91
GRADATIONPASSING NO.100 SIEVE %	80		86	70	83
GRADATIONPASSING NO.200 SIEVE %	76		83	65	80
SAND %	24		17	36	20
SILT %	34		40	28	32
CLAY % (.005 mm)	 				
CHAI & (.005 mm)	42	·	43	37	48
LIQUID LIMIT %	51 .		<u> </u>	54	47.
PLASTICITY INDEX %	27			25	25
UNCONFINED COMPRESSION STRENGTH	3.45		1.69	4.37	2.29
STANDARD DRY DENSITY					
AASHTO T-99 PCF	100.8	84.8	100.6	118.8	104.5
OPTIMUM MOISTURE %	22.3	29.0	18:4	13.7	22.7

CLIENT:_	HARZA ENVIRONMENTAL SERVICES	_	JOB NUMBER:	25,441
	· · · · · · · · · · · · · · · · · · ·	<u>. </u>	DATE:	JUNE 23, 1988
PROJECT:	ARLINGTON HEICHTS LANDFILL .			

			·	· · · · · · · · · · · · · · · · · · ·	·
LAB NUMBER					
BORING NUMBER	SB-3	SB-3	SB-4	SB-4	GW-1
SAMPLE NUMBER	4 TOP	4 BOTTOM	2	4	7
DEPTH IN FEET	8.5-10	8.5-10	3.5-5	8.5-10	12-14
HRB CLASSIFICATION & GROUP INDEX	OL_	CL	OL	CL	CL .
GRAIN SIZE CLASSIFICATION					
GRADATIONPASSING 1" SIEVE %					
GRADATIONPASSING 3/4" SIEVE %					100
GRADATIONPASSING 3/8" SIEVE %	<u></u>			100	91
GRADATIONPASSING NO.4 SIEVE %	·			98	91
GRADATIONPASSING NO.10 SIEVE %	·		100	96	89
GRADATIONPASSING NO.40 SIEVE %			97	85	81
GRADATIONPASSING NO.100 SIEVE %			93	74	75
GRADATIONPASSING NO.200 SIEVE %			91	70	72
SAND %			9	30	28
SILT %			42	35	34
CLAY %	· .		49	35	38
LIQUID LIMIT %			54	42	30
PLASTICITY INDEX %			32	23	16
*UNCONFINED COMPRESSION STRENGTH	2.00	2.69	1.77	2.36	
STANDARD DRY DENSITY					·
AASHTO T-99 PCF	98.6	112.8	95.4	111.2	
OPTIMUM MOISTURE %	22.1	17.9	28.4	17.6	15.6

CLIENT:	HARZA ENVIRONMENTAL SEI	RVICES	JOB NUMBER:	25,441
			DATE:	JUNE 27, 1988
PROJECT:	ARLINGTON HEIGHTS LANDI	TILL		

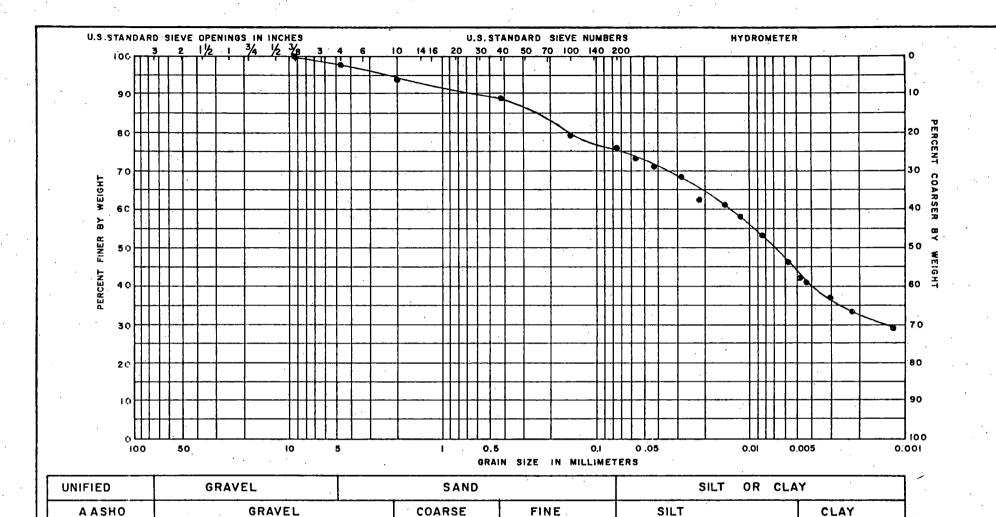
<u> </u>				,	,
LAB NUMBER		·			
BORING NUMBER	GW-1	GW-2	CW-2	GW-3	GW-3
SAMPLE NUMBER	17	6	9	. 8	10
DEPTH IN FEET	32-34	10-12	16-18	14-16	18-20
HRB CLASSIFICATION & GROUP INDEX			CL		CL
GRAIN SIZE CLASSIFICATION					
GRADATIONPASSING 1" SIEVE %					
GRADATIONPASSING 3/4" SIEVE %	100	100		100	
GRADATIONPASSING 3/8" SIEVE %	84	93		. 98	
GRADATIONPASSING NO.4 SIEVE %	69	87		94	
GRADATIONPASSING NO.10 SIEVE %	48	76		86	
GRADATIONPASSING NO.40 SIEVE %	17	24		74	
GRADATIONPASSING NO.100 SIEVE %	11	17		38	
GRADATIONPASSING NO.200 SIEVE %	. 9	14		28	
SAND %			·		
SILT %		·			
CLAY %					
	-				
LIQUID LIMIT %			34		. 27
PLASTICITY INDEX %	·		19	 	13
BEARING RATIO %				-	
STANDARD DRY DENSITY	• .		•		
AASHTO T-99 PCF		ļ	<u> </u>	 	
OPTIMUM MOISTURE %			18.1		12.4

CLIENT:_	HARZA ENVIRONMENTAL SERVICES	_ JOB NUMBER:	25,441
. <u>.</u>		_ DATE:	JUNE 23, 1988
	· · · · · · · · · · · · · · · · · · ·	_	
PROJECT:	ARLINGTON HEIGHTS LANDFILL		· · · · · · · · · · · · · · · · · · ·

LAB NUMBER					
BORING NUMBER	GW-4	GW-5	GW-5	CW-6	CW-6
SAMPLE NUMBER	11	6	10	14	21
DEPTH IN FEET	20-22	10-12	18-20	26-28	40-42
HRB CLASSIFICATION & GROUP INDEX	CL	CL .		CL	
GRAIN SIZE CLASSIFICATION					
GRADATIONPASSING 1" SIEVE %					
GRADATIONPASSING 3/4" SIEVE %					100
GRADATIONPASSING 3/8" SIEVE %	100				87
GRADATIONPASSING NO.4 SIEVE %	98				80
GRADATIONPASSING NO.10 SIEVE %	92				69
GRADATIONPASSING NO.40 SIEVE %	.86		100		45
GRADATIONPASSING NO.100 SIEVE %	80		90		14
GRADATIONPASSING NO.200 SIEVE %	. 77		39		10
SAND %	23				
SILT %	38				
CLAY % .005 mm	39				
LIQUID LIMIT %	28	27		24	
PLASTICITY INDEX %	14	13		11	
BEARING RATIO %					
STANDARD DRY DENSITY		}			
AASHTO T-99 PCF			ļ		_
OPTIMUM MOISTURE %	15.2	15.0		13.8	

CLIENT:_	HARZA ENVIRUNMENTAL SERVICES	`	JOB NUMBER:	ER: 25,441		
			DATE:	JUNE 23, 1988		
<i>*</i>				•		
PROJECT:	ARLINGTON HEIGHTS LANDFILL					
						

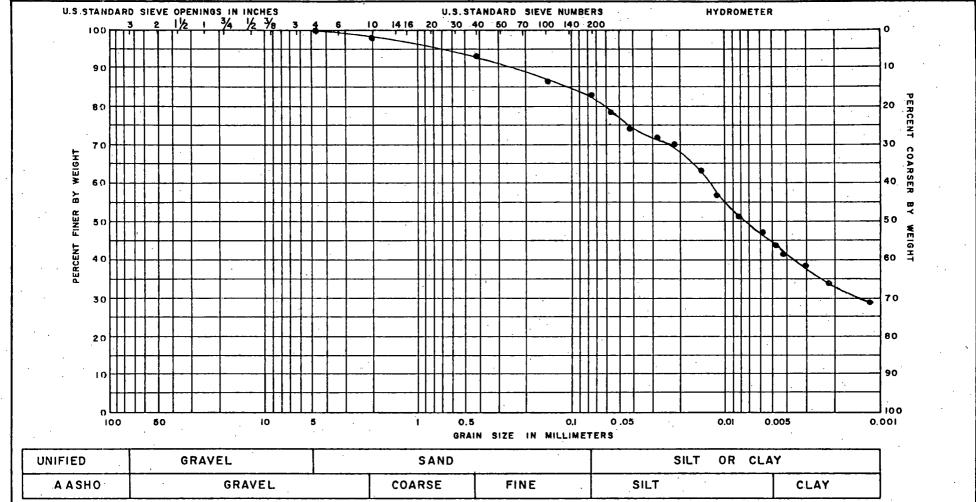
LAB NUMBER		•			
BORING NUMBER	GW-7	GW-7	GW-8	GW-8	
SAMPLE NUMBER	. 15	23	5	10	
DEPTH IN FEET	28-30	44-46	8-10	18-19.5	
HRB CLASSIFICATION & GROUP INDEX	CL	CL	CL		
GRAIN SIZE CLASSIFICATION					
GRADATIONPASSING 1" SIEVE %				100	
GRADATIONPASSING 3/4" SIEVE %	4.			88	
GRADATIONPASSING 3/8" SIEVE %	,	100		70	
GRADATIONPASSING NO.4 SIEVE %		97		57	
GRADATIONPASSING NO.10 SIEVE %		95		40	
GRADATIONPASSING NO.40 SIEVE %	•	89	<u> </u>	15	
GRADATIONPASSING NO.100 SIEVE %		84		10	
GRADATIONPASSING NO.200 SIEVE %		81		8	
SAND %			: .		
SILT %	·			<u> </u>	
CLAY %		<u> </u>			
LIQUID LIMIT %	26	,	54		
PLASTICITY INDEX %	12		32		
BEARING RATIO %					
STANDARD DRY DENSITY					
AASHTO T-99 PCF					
OPTIMUM MOISTURE %	17.8		17.5		



NUMBER	DEPTH	W	WL	WP	CLASSIFICATION
SB-1					
ST-1	1-2.5	22.3			

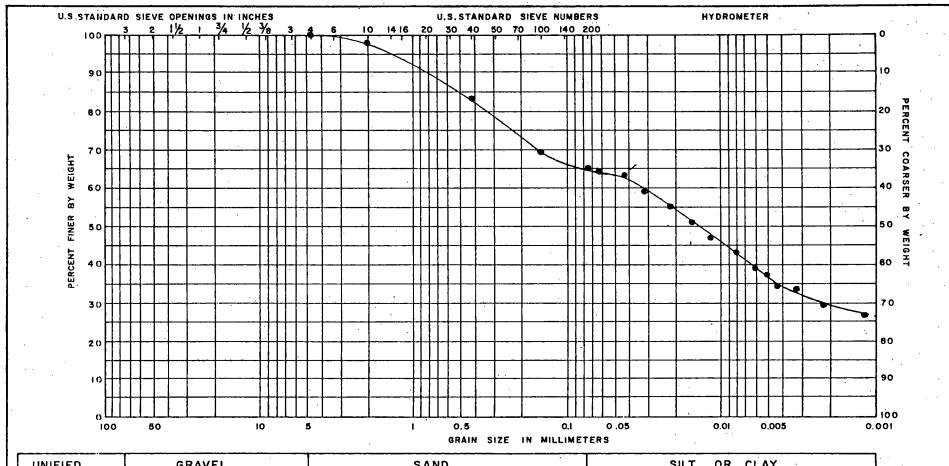
TESTING SERVICE CORPORATION
457 EAST GUNDERSEN DRIVE
CAROL STREAM, ILLINOIS 60188

L - 25,441



NUMBER	DEPTH	w	w	w _P	CLASSIFICATION
SB-2	1-2½ TOP	,			
	_				

TESTING SERVICE CORPORATION
457 EAST GUNDERSEN DRIVE
CAROL STREAM, ILLINOIS 60188
L - 25,441

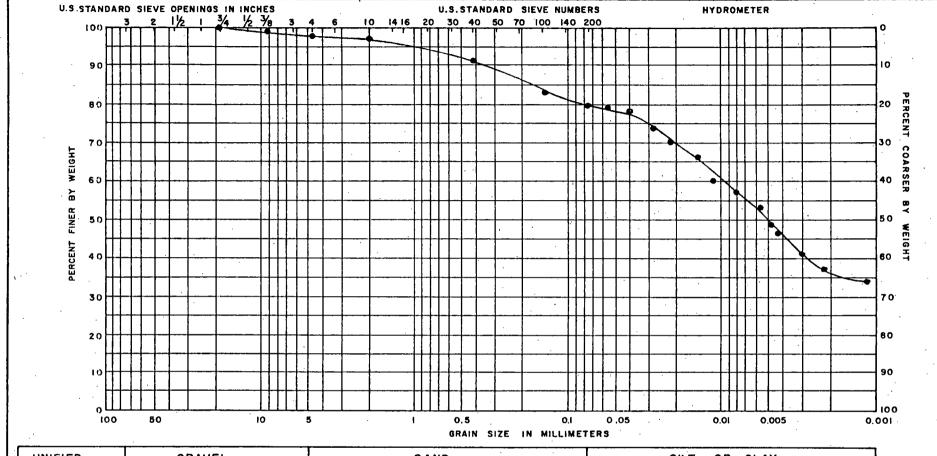


UNIFIED	GRAVEL	SAND	,	SILT	OR CLAY
A ASHO	GRAVEL	COARSE	FINE	SILT	CLAY

NUMBER	DEPTH	w	w_	W P	CLASSIFICATION
SB-2	1-2.5 BOTTO	1			·
				-	

TESTING SERVICE CORPORATION
457 EAST GUNDERSEN DRIVE
CAROL STREAM, ILLINOIS 60188

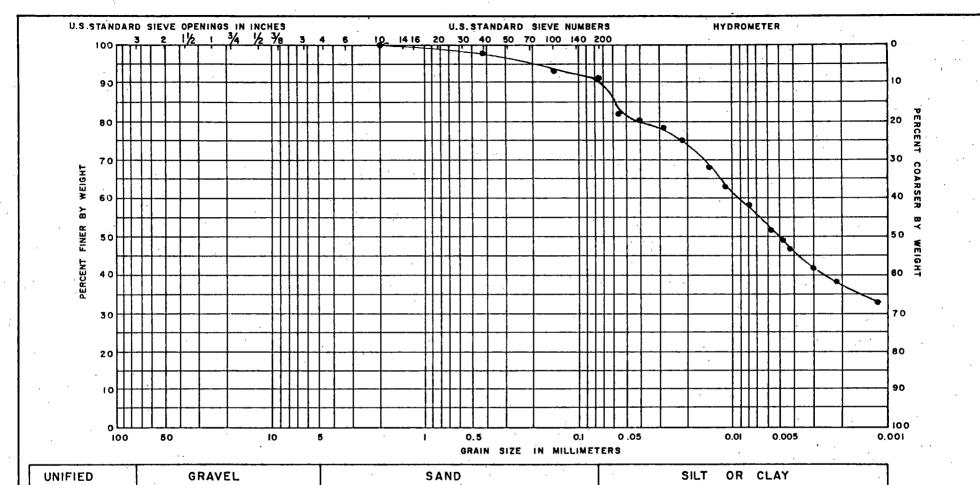
L - 25,441



UNIFIED	GRAVEL	SAND		SILT OR	CLAY
A ASHO	GRAVEL	COARSE	FINE	SILT	CLAY

DEPTH.	w	w_	W P	CLASSIFICATION
3-51/2				
	<u> </u>			
				·
:				
	 		 	

TESTING SERVICE CORPORATION 457 EAST GUNDERSEN DRIVE CAROL STREAM, ILLINOIS 60188 L - 25,441



FINE

COARSE

NUMBER	DEPTH	w	WL	W _P	CLASSIFICATION
SB-4					
ST-2	3.5-5	28.4	54	22	OL .
				·	

GRAVEL

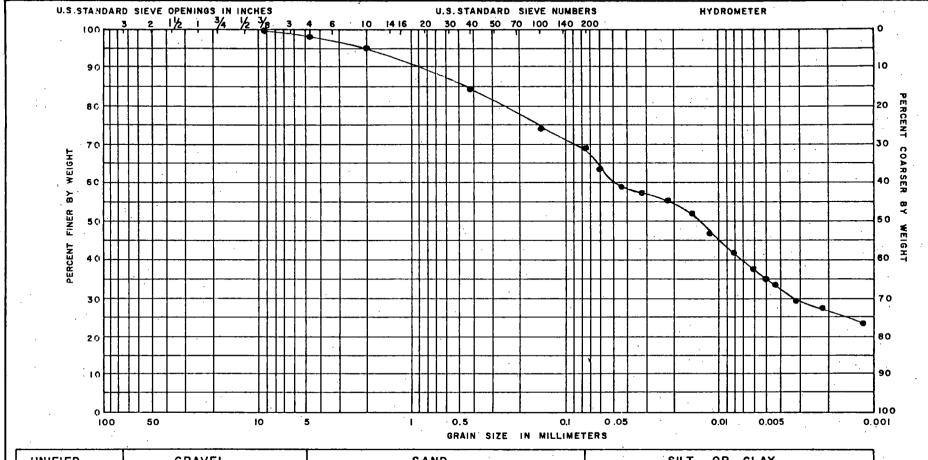
A ASHO

TESTING SERVICE CORPORATION
457 EAST GUNDERSEN DRIVE
CAROL STREAM, ILLINOIS 60188

CLAY

L - 25,441

SILT

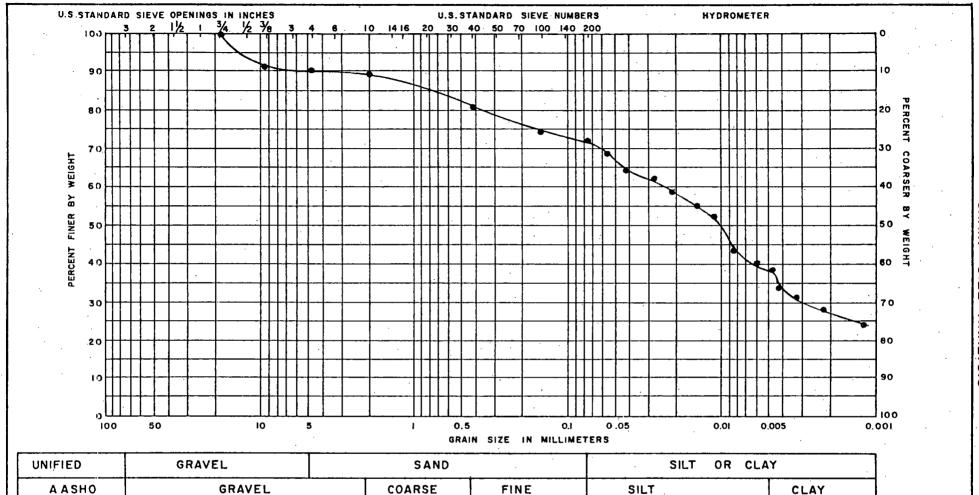


UNIFIED	GRAVEL	SAND		SILT (OR CLAY
A A SHO	GRAVEL	COARSE	FINE	SILT	CLAY

NUMBER	DEPTH	w	.w L	W P	CLASSIFICATION
SB-4					
ST-4	8.5-10	17.6	4.2	19	CL
		<u> </u>			

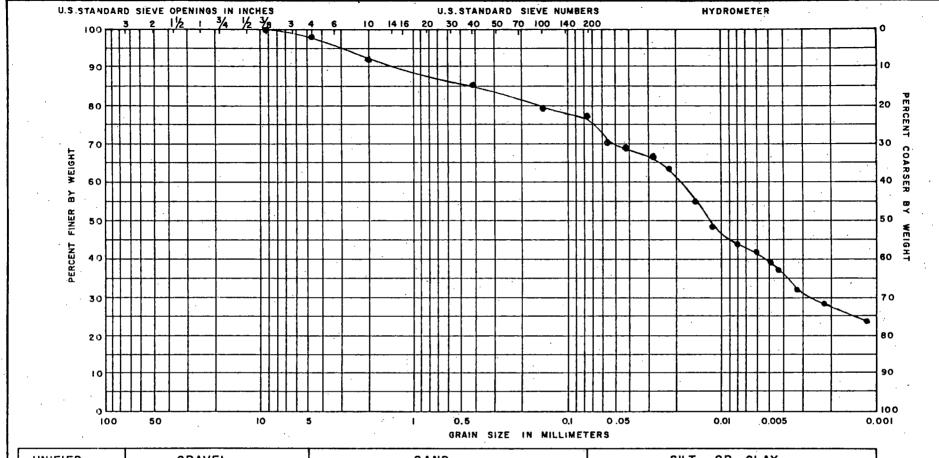
TESTING SERVICE CORPORATION
457 EAST GUNDERSEN DRIVE
CAROL STREAM, ILLINOIS 60188
L - 25,441

25441 iji.



NUMBER	DEPTH	w	WL	WP	CLASSIFICATION	
GW-1						
S-7 .	12-14	.15.6	30	14	CI.	

TESTING SERVICE CORPORATION
457 EAST GUNDERSEN DRIVE
CAROL STREAM, ILLINOIS 60188
L - 25,441

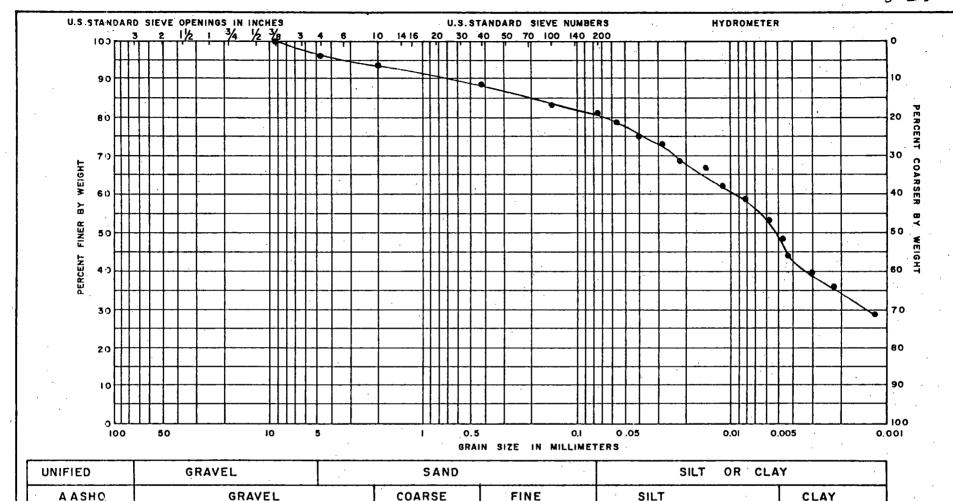


UNIFIED	GRAVEL	SAND		SILT 0	R CLA	Υ
A A SHO	GRAVEL	COARSE	FINE	SILT		CLAY

NUMBER	DEPTH	w	w_	W P	CLASSIFICATION
GW-4					
S-11 ·	20-22	15.2	28	14	CL
<u> </u>		<u> </u>			

TESTING SERVICE CORPORATION
457 EAST GUNDERSEN DRIVE
CAROL STREAM, ILLINOIS 60188

L - 25,441



NUMBER	DEPTH	. W	WL	.W _P	CLASSIFICATION
GW-7	·				
S-23	44-46	17.5	<u> </u>		
	,		-		
		-	1		

TESTING SERVICE CORPORATION
457 EAST GUNDERSEN DRIVE
CAROL STREAM, ILLINOIS 60188
L - 25,441

Appendix C

Laboratory Groundwater Analysis Reports



!	2621 Ridgepoint	ŀ
	Austin TX 78754	
	512-928-8905	

850 West Bartlett Rd.
 Bartlett IL 60103
 312-289-3100

222 South Morgan Chicago IL 60607 312-666-4469 ☐ 3548 35th St.
Rockford IL 61109
815-874-2171

CHAIN OF CUSTODY RECORD

Client 4	RLINGTON HEIGHTS U	AN DALC	Project	t	20		OB
Sampler(s)			<u> </u>		<u>_</u> e		Remarks
Number	Sampling Location	Date	Time	Sala		0 / V	Remarks
20906W-1	ARUNGTON HEIGHTS	5-12	2:15	1	- 1	2) p #
2090 6 w-2	HELINGTON HEIGHTS	ч	4:40	>	K	2	CONDUCTIVITY
090-610-3	ARLINGTON HEIGHTS	4	12:10	\	×	2	Ca
1090 GW 4	ARLINGTON HEIGHTS	4	3:20	, ,	×	2	Mg
090610-5	ARLINGTON HEIGHTS	4	1:00	,	×	2	Na
0906W-6	ARLINGTON HEIGHTS	4	11:80		×	2	K
090GW-7	ACLINGTON HEIGHTS	4	10:00	,	x	2	Sulfate
	ARLINGTON HEIGHTS	4	4:00	,	×	2	a
090 SW-1	ARLINGTON HEIGHTS	4	2:20	,	×	2	ALLIALINITY AS
							BICARBONATE
			-				ONE BOTTLE FROM EACH
							BORING - CAR. FILTER

Relinquished By	Heceived By	Date	lime
JOHN PYRICH		٠.	
HARZA ENVIRONMENTAL SERVICES		·	
150 S. WACKER DR.			·
CHICAGO 16 60006			
Shipping Notes	Received For Aqualab By		
	Path Plage	5/13/88	1345



_	2621 Ridgepoint
	Austin TX 78754
	512-928-8905

850 West Bartlett Rd. Bartlett IL 60103 312-289-3100

222 South Morgan Chicago IL 60607 312-666-4469

3548 35th St. Rockford IL 61109 815-874-2171

CHAIN OF CUSTODY RECORD

Client AR	LINGTON HEIGHTS LAN	DFILL	Project	t		90 B
Sampler(s)				/.	<u>_</u> e/	Remarks
Number	Sampling Location	Date	Time	000	0 0 0 0	Remarks
2090 GW-1	BRUNGTON HEIGHTS	5-/2	2:18	×	2	
2090 GW-2		4	4:40	×	2	ONE BOTTLE FROM
10906W-3	<u>u</u>	4	12:10	X	(2	EACH WELL
0906W-4	//	4	3:20)	(2	1250 CC PLASTIC
09066-5	11	4	1:00	X	2	LABELLED HZSOQ)-
0906W-6	<u>u</u>	. 4	11:30	×	12	- FOR TOC
0906W-7	<u>u</u>	9	10:00	×	2	ONE BOTTLE PROM
2090GW-8	//	4	4:00	\	2	EACH WELL
090 \$ W-1	<i>[1</i>	4	2:20	>	(2	(802 GLASS) -
						- FOR TOX
·						

Relinquished By	Received By	Date	Time
JOHN PYRICH			
HARZA ENVIRONMENTAL SERVICES			
150 C. WACKER DR			
CHICAGO K 60606			
Shipping Notes	Received For Aqualab By		
	Patu Piagg	5/13/88	1375
# 140			



NET Midwest, Inc. Bartlett Division 850 West Bartlett Road Bartlett, IL 60103 Tel: (312) 289-3100

Tel: (312) 289-3100 Fax: 312-289-4180

Formerly: Aqualab, Inc.

ANALYTICAL REPORT

Mr. Bob Kewer HARZA ENGINEERING CO. 150 So. Wacker Drive Chicago IL 60606 06-21-88

Sample No.: 63098

Sample Description:

2090GW-2

2090 B

Date Taken: 05-12-88 1640

Date Received: 05-13-88 1345

Alkalinity, Bicarb. (CaCO3)	476.	mg/L
Chloride	140.	mg/L
Conductivity	1770.	umhos/cm
рН	7.43	units
Sulfate	205.	mg/L
Total Organic Carbon (TOC)	4.4	mg/L
Total Organic Halogens	0.013	mg/L
Calcium	118.	mg/L
Magnesium	85.0	mg/L
Potassium	26.2	mg/L
Sodium	83.0	mg/L

Whoteshed



Tel: (312) 289-3100 Fax: 312-289-4180

Formerly: Aqualab, Inc.

ANALYTICAL REPORT

Mr. Bob Kewer HARZA ENGINEERING CO. 150 So. Wacker Drive Chicago IL 60606 06-21-88

Sample No.: 63099

Sample Description:

2090GW-3

2090 B

Date Taken: 05-12-88 1210

Date Received: 05-13-88 1345

Alkalinity, Bicarb.(CaC03)	478.	mg/L
Chloride	254.	mg/L
Conductivity	2530.	umhos/cm
рН	6.93	units
Sulfate	369.	mg/L
Total Organic Carbon (TOC)	12.2	mg/L
Total Organic Halogens	0.093	mg/L Cl-
Calcium	238.	mg/L
Magnesium	90.0	mg/L
Potassium	5.33	mg/L
Sodium	161.	mg/L

W. Motter die



Tel: (312) 289-3100 Fax: 312-289-4180

Formerly: Aqualab, Inc.

ANALYTICAL REPORT

Mr. Bob Kewer HARZA ENGINEERING CO. 150 So. Wacker Drive Chicago IL 60606 06-21-88

Sample No.: 63100

Sample Description:

2090GW-4

2090 B

Date Taken: 05-12-88 1520

Date Received: 05-13-88 1345

Alkalinity, Bicarb. (CaCO3)	294.	mg/L
Chloride	82.	mg/L
Conductivity	1100.	umhos/cm
рН	7.42	units
Sulfate	131.	mg/L
Total Organic Carbon (TOC)	2.0	mg/L
Total Organic Halogens	<0.005	mg/L
Calcium	89.3	mg/L
Magnesium	47.0	mg/L
Potassium	3.95	mg/L
Sodium	27.6	mg/L

W. Ustreches



Tel: (312) 289-3100 Fax: 312-289-4180

Formerly: Aqualab, Inc.

ANALYTICAL REPORT

Mr. Bob Kewer HARZA ENGINEERING CO. 150 So. Wacker Drive Chicago IL 60606 06-21-88

Sample No.: 63101

Sample Description:

2090GW-5

2090 B

Date Taken: 05-12-88 1300

Date Received: 05-13-88 1345

Alkalinity, Bicarb. (CaCO3)	138.	mg/L
Chloride	24.	mg/L
Conductivity	934.	umhos/cm
рН	7.90	units
Sulfate	239.	mg/L
Total Organic Carbon (TOC)	2.4	mg/L
Total Organic Halogens	<0.005	mg/L
Calcium	81.6	mg/L
Magnesium	26.7	mg/L
Potassium	3.25	mg/L
Sodium	62.0	mg/L

WWOWELLES William H. Mottashe



NET Midwest, Inc. **Bartlett Division** 850 West Bartlett Road Bartlett, IL 60103 Tel: (312) 289-3100

Fax: 312-289-4180

Formerly: Aqualab, Inc.

ANALYTICAL REPORT

Mr. Bob Kewer HARZA ENGINEERING CO. 150 So. Wacker Drive Chicago IL 60606

06-21-88

Sample No.: 63102

Sample Description:

2090GW-6

2090 B

05-12-88 1130 Date Taken:

Date Received: 05-13-88 1345

Alkalinity, Bicarb. (CaC03)	388.	mg/L
Chloride	182.	mg/L
Conductivity	1430.	umhos/cm
рН	7.16	units
Sulfate	84.	mg/L
Total Organic Carbon (TOC)	2.1	mg/L
Total Organic Halogens	0.009	mg/L
Calcium	137.	mg/L
Magnesium	48.0	mg/L
Potassium	5.35	mg/L
Sodium	61.0	mg/L



Tel: (312) 289-3100 Fax: 312-289-4180

Formerly: Aqualab, Inc.

ANALYTICAL REPORT

Mr. Bob Kewer HARZA ENGINEERING CO. 150 So. Wacker Drive Chicago IL 60606 06-21-88

Sample No.: 63103

Sample Description:

2090GW-7

2090 B

Date Taken: 05-12-88

Date Received: 05-13-88 1345

Alkalinity, Bicarb. (CaCO3)	230.	mg/L
Chloride	128.	mg/L
Conductivity	1040.	umhos/cm
рН	7.43	units
Sulfate	75.	mg/L
Total Organic Carbon (TOC)	1.8	mg/L
Total Organic Halogens	0.018	mg/L
Calcium	93.8	mg/L
Magnesium	48.3	mg/L
Potassium	4.79	mg/L
Sodium	46.3	mg/L

W. Wotteshed



Tel: (312) 289-3100 Fax: 312-289-4180

Formerly: Aqualab, Inc.

ANALYTICAL REPORT

Mr. Bob Kewer HARZA ENGINEERING CO. 150 So. Wacker Drive Chicago IL 60606 06-21-88

Sample No.: 63104

Sample Description:

2090GW-8

2090 B

Date Taken: 05-12-88 1600

Date Received: 05-13-88 1345

680.	mg/L
210.	mg/L
2200.	umhos/cm
6.85	units
59.	mg/L
20.6	mg/L
0.056	mg/L
157.	mg/L
66.0	mg/L
37.2	mg/L
163.	mg/L
	2200. 6.85 59. 20.6 0.056 157. 66.0 37.2



Tel: (312) 289-3100 Fax: 312-289-4180

Formerly: Aqualab, Inc.

ANALYTICAL REPORT

Mr. Bob Kewer HARZA ENGINEERING CO. 150 So. Wacker Drive Chicago IL 60606 06-21-88

Sample No.: 63105

Sample Description:

2090SW-1

2090 B

Date Taken:

05-12-88 1420

Date Received:

05-13-88 1345

Alkalinity, Bicarb. (CaCO3)	138.	mg/L
Chloride	72.	mg/L
Conductivity	659.	umhos/cm
рН	8.15	units
Sulfate	62.	mg/L
Total Organic Carbon (TOC)	5.1	mg/L
Total Organic Halogens	0.006	mg/L
Calcium	59.7	mg/L
Magnesium	21.4	mg/L
Potassium	3.90	mg/L
Sodium	39.3	mg/L

William H Mott



NET Midwest, Inc. Bartlett Division 850 West Bartlett Road Bartlett, IL 60103 Tel: (312) 289-3100 Fax: 312-289-4180

Formerly: Aqualab, Inc.

ANALYTICAL REPORT

Mr. Bob Kewer HARZA ENGINEERING CO. 150 So. Wacker Drive Chicago IL 60606 06-21-88

Sample No.: 63097

Sample Description:

2090GW-1

2090 B

Date Taken: 05-12-88 1415

Date Received: 05-13-88 1345

Alkalinity, Bicarb. (CaCO3)	412.	mg/L
Chloride	112.	mg/L
Conductivity	1540.	umhos/cm
рН	7.42	units
Sulfate	219.	mg/L
Total Organic Carbon (TOC)	4.7	mg/L
Total Organic Halogens	0.080	mg/L
Calcium	159.	mg/L
Magnesium	68.0	mg/L
Potassium	7.58	mg/L
Sodium	52.5	mg/L